STORMWATER MANAGEMENT REPORT FOR BASEBALL FACILITIES AT BROOK RUN PARK

CITY OF DUNWOODY DEKALB COUNTY, GEORGIA

March 31, 2017 Revised May 3, 2017 Revised May 18, 2017 Revised July 11, 2017



SKYLINE Engineering & Construction 6755 Peachtree Industrial Blvd. Suite 240 Atlanta GA 30360 Office: (888) 706-0661

Fax: (770) 217-7530

EXECUTIVE SUMMARY

The objective of the following report is to provide an overview of the hydrologic impact that will result from developing the subject property located in the City of Dunwoody. In general, the primary hydrologic impact of development is an increase in peak storm water runoff rates from the site. Left unmitigated, this increase in peak runoff rates has the potential of increasing downstream flooding. This report provides an assessment of a proposed on-site storm water management facilities in a manner consistent with the current drainage policies and regulations of the City of Dunwoody's Post-Development Stormwater Management Ordinance.

SITE DESCRIPTION

PRE-DEVELOPED CONDITIONS

The Baseball Facilities at Brook Run Park project is located 4664 N Peachtree Road in Dunwoody, Georgia. The Baseball Facility for the park is located at the southwest corner of the intersection of North Peachtree Road and Barclay Drive and consists of approximately 7.5 acres. For the purpose of the following analysis this 7.5 acre tract is considered the "onsite" area and all other portions of the property is considered "offsite". Currently the 7.5 acre site contains an open grassed field with approximately 2.8 acres of wooded area and sloping terrain.

The majority of the 7.5 acre site, discharges into an existing 54" culvert under Barclay Drive into an unnamed tributary to Nancy Creek Tributary A. The inlet to the existing culvert is referred to as Study Point A in the following analysis. The following analysis divides the drainage areas combining at Study Point A into three sub-basins; A1, A2 and A3. Sub-basin A1 consists of 7 acres of the site which drains to Study Point A via existing storm drain bisecting the property. Sub-basin A2 consists of stormwater discharge from an existing detention facility for Peachtree Charter Middle School. The existing detention facility is modeled in the following analysis as per construction documents by Eberly & Associates. Discharge from the existing detention facility drains directly into the headwater pool at Study Point A. Sub-basin A3 consists of additional upstream offsite areas which also drain to Study Point A via existing storm drain bisecting the property.

Please refer to the Pre-Developed Drainage Basin Map.

POST-DEVELOPED CONDITIONS

Proposed land disturbance to the 7.5 acre site includes demolition of all existing storm drains as shown on the associated construction documents, construction of 2 baseball fields with artificial turf, accessory buildings and parking areas for the baseball facilities, installation of utilities, landscaped areas, and sidewalks. Additionally, the following analysis has included a future building addition to an existing school building which will include approximately 0.50 acres of disturbed area and 12,200 sf of impervious area. Stormwater Runoff from the addition will ultimately discharge into the proposed Stormwater Pond. The proposed Stormwater Pond is located just north of Study Point A to provide attenuation of all storm events at Study Point A before stormwater discharges the property.

Please refer to the Post-Developed Conditions Drainage Basin Map.

STORMWATER SIZING CRITERIA

WATER QUALITY PROTECTION

As stated in Section 2.2.4.1 of the 2016 Georgia Stormwater Manual the Water Quality goal can be accomplished either through runoff reduction or water quality treatment or some combination of the two. The attached Georgia Stormwater Management Manual Stormwater Quality Site Development Review Tool Spreadsheet calculates the water quality volume required (8,760 ft³) to provide 80% TSS reduction with proposed improvements to the 7.5 acre site and the additional 0.5 acres of the future school addition's disturbed area.

Proposed improvements to the site will include a Vortech Stormwater treatment devices by Contech in proposed storm drain inlets to provide 7,407 ft³ of volume. Please refer to the construction documents for design details and calculations.

Water quality volume beneath the five acres of proposed artificial turf is provided within the turf's gravel base. The minimum depth of gravel required to provide 1,089 ft³ is 2 inches based on a 40% void ratio for the stone.

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1,089 \text{ ft}^3 / 0.40 = 2723 \text{ ft}^3

2723 \text{ ft}^3 / (5.0 \text{ ac.} * 43,560 \text{ ft}^2/\text{ac}) = 0.13 \text{ ft} = 1.56 \text{ inches}
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STREAM CHANNEL PROTECTION

Stream Channel Protection has been provided in the proposed Stormwater Pond as per Section 2.2.4.2 of the 2016 Georgia Stormwater Management Manual. Please refer to the attached calculations.

OVERBANK FLOODING PROTECTION

Overbank Flooding Protection requirements for the site has been met by attenuating the post-developed peak flows for the 1, 2, 5, 10 and 25 year storm events at Study Point A in the proposed Stormwater Pond.

EXTREME FLOODING PROTECTION

Extreme Flooding Protection requirements for the site has been met by attenuating the post-developed 100 year storm event at Study Point A in the proposed Stormwater Pond.

HYDROLOGIC METHODOLOGY

Hydrologic data of the following evaluation is based on field reconnaissance of the property, a watershed delineation of the existing topography utilizing field run surveys and the DeKalb County GIS website. This data is then used to compute peak storm water runoff rates for the city's required design recurrence intervals of 1,2, 5, 10, 25, 50 and 100 years. The SCS Hydrologic Methodology was applied using the Type II rainfall distribution for the 24 hour storm in Atlanta, Georgia and curve number values area based on GSMM Table 2.15-1 for type "B" soils.

Time of concentration values for pre-developed and post-developed conditions are calculated using the SCS TR-55 methodology. Analysis for all storm events was performed using Hydraflow Hydrographs Extension 2009, hydrology & hydraulics software program, version 6.066 by *Autodesk, Inc.*

SUMMARY OF RESULTS

As shown in the table below peak rates of flow and ponding elevations for all storm events have been reduced to meet pre-developed conditions with the design of the proposed Stormwater Pond.

| | | Stud | y Pt. A | | |
|--------------------|---------------------------------|---------------------|---------------------------------|----------------------|-------------------|
| Storm Frequency | Pre- Development | Pre- Development | Post- Development | Post- Development | Peak Rate of Flow |
| (yr) | Conditions Discharge Rate (cfs) | Ponding El. (ft) | Conditions Discharge Rate (cfs) | Ponding El. (ft) | Decrease |
| 1 | 25.44 | 970.49 | 24.43 | 970.46 | 4.0% |
| 2 | 40.90 | 971.42 | 37.91 | 970.92 | 7.3% |
| 5 | 58.02 | 971.53 | 52.59 | 971.37 | 9.4% |
| 10 | 76.32 | 972.04 | 70.01 | 971.86 | 8.3% |
| 25 | 105.91 | 972.91 | 100.29 | 972.72 | 5.3% |
| 50 | 132.72 | 974.01 | 127.28 | 973.79 | 4.1% |
| 100 | 158.40 | 975.30 | 157.07 | 975.23 | 0.8% |

| | Proposed S | Stormwater P | ond |
|----------------------------|--------------------------------------|---------------------------|------------------------------|
| Storm Frequency (yr) | Routed Discharge Rate (cfs) | Storage Volume (cf) | Ponding Elevation (ft) |
| 1 | 0.54 | 37,543 | 981.26 |
| 2 | 1.57 | 45,111 | 982.20 |
| 5 | 3.69 | 52,005 | 983.01 |
| 10 | 7.00 | 60,023 | 983.94 |
| 25 | 12.21 | 71,083 | 985.13 |
| 50 | 16.54 | 79,065 | 985.98 |
| 100 | 30.58 | 83,726 | 986.45 |

DOWNSTREAM ANALYSIS

As stated previously, stormwater from the 7.5 acres site discharges into an existing 54" culvert under Barclay Drive and into an unnamed tributary to Nancy Creek Tributary A. Stormwater then travels approximately 400 linear feet through Brook Run Park into an existing culvert under Georgia Way S. As shown on the table below, all peak rates of flow at this location are reduced with proposed improvements to the site.

| | Georgia Wa | y S Study Pt. | |
|----------------------------|--|---|----------------------------------|
| Storm Frequency (yr) | Pre- Development Conditions Discharge Rate (cfs) | Post- Development Conditions Discharge Rate (cfs) | Peak Rate of Flow Decrease |
| 1 | 40.71 | 39.01 | 4.2% |
| 2 | 67.18 | 62.49 | 7.0% |
| 5 | 96.31 | 88.28 | 8.3% |
| 10 | 127.85 | 116.00 | 9.3% |
| 25 | 171.26 | 155.65 | 9.1% |
| 50 | 205.95 | 187.86 | 8.8% |
| 100 | 243.65 | 224.11 | 8.0% |

Stormwater from the existing culvert under Georgia Way S then travels approximately 1120 linear feet to the Downstream Study Point for the project. The total drainage area at this location is approximately 79.8 acres; therefore the 7.5 acre site is approximately 9% of the total drainage area at this location. As shown on the table below, all peak rates of flow at this location are reduced with proposed improvements to the site.

| | Downstrea | m Study Pt. | | | | |
|----------------------------|--|---|----------------------------------|--|--|--|
| Storm Frequency (yr) | Pre- Development Conditions Discharge Rate (cfs) | Post- Development Conditions Discharge Rate (cfs) | Peak Rate of Flow Decrease | | | |
| 1 | 45.83 | 44.04 | 3.9% | | | |
| 2 | 78.00 | 73.31 | 6.0% | | | |
| 5 | 114.53 | 106.42 | 7.1% | | | |
| 10 | 153.93 | 142.00 | 7.8% | | | |
| 25 | 208.88 | 192.97 | 7.6% | | | |
| 50 | 253.81 | 235.96 | 7.0% | | | |
| 100 | 299.36 | 282.80 | 5.5% | | | |

Georgia Stormwater Management Manual Stormwater Quality Site Development Review Tool Version 2.2

| ook Run Pa | 9224 IN PERCHIEF KORU | nstitutional, Public & Semi Public | | 8.00 | Total Treated Area (ac): 6.79 Total Untreated Area (ac): 1.21 | I (ac) | 6.21 | 0.00 0.00 0.00 0.00 | 0.00 0.00 0.00 0.00 | 0.00 | 00.0 | 00:00 | 0.00 | OLAL 1.79 6.21 0.00 | Target Runoff Reduction Volume Achieved? No | | | Kunott Reduction Volume Acnieved (ct) 908 Total Tardet Water Quality Volume (cf) 8.760 | | 9 | | |
|--|---|------------------------------------|--------------|--------------------------------------|--|--------|-------------|---------------------|---------------------|---------------|------|-----------------------|------|---------------------------|---|--------------|-----|---|------------|-------------------|-------------------------|--|
| General Information Date Submitted: Permit Number: | Developer Contact: Phone Number: Name of Engineer(s): | Maintenance | Site Summary | ,000 ,000 | %00 80 80 80 | | %G onpəy | 1 227 | 80% | 0% | | 700% | ò | %08 %08 | •M †•8 | ыт яя 40% | 70% | 15 | DB 1 | Official Use Only | Cond | |
| on ted: ber: | ontact: ber: gineer(s): | Maintenance Responsibility: | | Total | | | | | %0 %0 | - 7 80 | 1 | | | | | | | %0 %0 | L DB2 DB3 | Λ | Conditions of Approval: | |
| 7/11/2017 | Skylir | | | Total Suspended Solids (TSS) Removal | | | | | %0 | 2 / 20 / | | Runo | | | | | | %0 | 3 DB 4 | | roval: | |
| 2017 | Skyline City of Dunwoody | City of Dunwoody | | olids (TSS) Ro | | | | | %0 %0 | - 7 an | | Runoff Reduction (RR) | | | | | | %0 %0 | DB 5 DB 6 | | | |
| Ш | yboown | , woody | | emoval | | | | | %0 | - 7 8 0 | | (RR) | | | | | | %0 | DB 7 | | | |
| Ш | | | | | | | | | %0 | - 880 | | | | | | | | %0 %0 | DB 8 DE | | | |
| | П | | | | | | | | %0 %0 | - 01 01 a0 | , | | | | | | | %0 %0 | DB 9 DB 10 | | | |
| | | | | | | | | | | | | ſ | | | | | | | | | | |

Stormwater Quality Site Development Review Tool, v2.2 Georgia Stormwater Management Manual

| data input cells | calculation cells constant values | | | CN HSG D CN Total %Cover (acres) | 80 4.70 | 77 3.30 | 00.00 | %0 00.0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 | 0000 | | 8.00 100% | Impervious (ac) 0.00 Weighted CN 59 Potential Max Soil Retention, S _{pre} (in) 7.09 | | CN HSG D CN Total % Cover | | 80 1.21 | 89 5.00 | 0 0.00 0 000 0 | | 00.00 | | 00.0 | Impervious (ac) 1.79 Rv 0.25 Weighted CN 81 Potential Max Soil Retention, S _{cost} (in) 2.42 | | Scenario 3: Soil Restoration *See the GSMM Volume 2, Section 4.23 for more information. | Check the box if a portion of the post-developed area employs <u>soil restoration</u> and is protected by a conservation easement or equivalent form of protection. | Area (ac) of development with restored soils and protected by a conservation Note: The green cell will unlock if the Scenario seasement or equivalent form of protection. | *See the GSMM Volume 2, Section 4.22 and 4.23 for more information. | Check the box if the same portion of the post-developed area employs site reforestation/revegetation and soil <u>restoration,</u> and is protected by a conservation easement or equivalent form of protection. | Area (ac) with restored soils in a reforested & revegetated area and protected Note: The green cell will unlock if the Scenario by a conservation easement or equivalent form of protection. | |
|---|-----------------------------------|-----------|--|----------------------------------|---|------------------------|--------------------------|---|--------------------------|-------|-----------|--|--|---------------------------|------------|---|---|--------------------------|--------------------------|--------------------------|------|------|--|----------------------------------|---|---|--|--|---|--|--|
| | | Site Data | | CN HSG C (acres) | | 55 | 0 | 0 | | | 0.00 | ă | | CN HSG C | | 61 | 79 | 0 | 0 | | | 0.00 | <u>o</u> | Conservation Area Credits | Scenario 3: 5 | | | Scenario 4: 5 | | | 0.00 |
| ark | | | | HSG B (acres) | | 3.30 | | | | | 8.00 | | | HSG B (acres) | 1.79 | | | | | | CO | 0.00 | | Conserva | | ivalent form of | lock if the Scenario 1 box | | rotected by a | lock if the Scenario 2 box | Total Conservation Area Credit (acres) |
| lities at Brook Run P | | | e's Disturbed Area | HSG* A CN (acres) | | 30 | 0 | | | | 0.00 | | ite's Disturbed Area | HSG A CN | | 39 | 89 | 0 | 0 | | 00 0 | 0.00 | | | r more information. | vation easement or equ | Note: The green cell will unlock if the Scenario 1 box above is checked | for more information. | on/revegetation and is pi | Note: The green cell will unlock if the Scenario 2 box above is checked | Total Conservation |
| Development Name: Baseball Facilities at Brook Run Park | Drainage Basin Name: | | Indicate Pre-Development Land Cover and Runoff Curve Numbers in the Site's Disturbed | | Open space - Good condition (grass cover > 75%) | Woods - Good Condition | Select a land cover type | Select a land cover type | Select a land cover type | Other | Total | 'HSG = hydrologic soil group | Indicate Post-Development Land Cover and Runoff Curve Numbers in the Site's Disturbed Area | Cover Type | Impervious | Open space - Good condition (grass cover > 75%) | Open space - Poor condition (grass cover < 50%) | Select a land cover type | Select a land cover type | Local Jurisdiction Input | | 1018 | | | Scenario 1: Natural Conservation Area *See the GSMM Volume 2, Section 2.3.3.3 for more information. | Check the box if a portion of the post-developed area is protected by a conservation easement or equivalent form of protection. | Area (ac) of development protected by a conservation easement or equivalent form of protection. | Scenario 2: Site Reforestation/Revegetation *See the GSMM Volume 2, Section 4.22 for more information. | Check the box if a portion of the post-developed area employs <u>site reforestation/revegetation</u> and is protected by a conservation easement or equivalent form of protection. | Area (ac) of development reforested/revegetated and protected by a conservation easement or equivalent form of protection. | |

Stormwater Quality Site Development Review Tool, v2.2 Georgia Stormwater Management Manual

Development Name: Baseball Facilities at Brook Run Park
Drainage Basin Name:

data input cells
calculation cells
constant values

Water Quality Goals

Target Runoff Reduction Storm (in) 1.00

Total Site Area for Water Quality Volume (acres)
Target Runoff Reduction Volume (cf)
Target Water Quality Volume (cf)

Select BMPs for Runoff Reduction and Water Quality

| | | Area Dr. | Area Draining to Each BMP | | RR Conveyance | RR Conveyance | | | Ru | Runoff Reduction Calculations | Calculations | | | WQ Calculations | ulations |
|--------|------------------------|-------------------------------------|--|-------------------------|-------------------------------------|--------------------------------------|--------------------|---|--|---|-----------------------|---------------------|--------------------------------|---|-------------------------------|
| | | On-site Pervious Area (acres) | On-site Impervious Area (acres) | Offsite Area (acres) | Storage Volume Provided by BMP (cf) | Volume Provided by BMP (cf) | Down-stream BMP | RR Volume from Direct Drainage (cf) | RR Volume from Upstream Practices (cf) | Total RR Volume Received by BMP (cf) | Runoff Reduction % | RR Achieved (cf) | Remaining RR Volume (cf) | WQ _v from Direct Drainage (cf) | Effective TSS Removal % |
| BMP 1 | Proprietary System | | 1.79 | | 7,407 | | | 6,173 | 0 | 6,173 | %0 | 0 | 6,173 | 7,407 | %08 |
| BMP 2 | Infiltration Trench | 5.00 | | | 1,089 | | | 806 | 0 | 806 | 100% | 806 | 0 | 1,089 | 100% |
| BMP 3 | Select a BMP | | | | | | | 0 | 0 | 0 | N/A | 0 | 0 | 0 | N/A |
| BMP 4 | Select a BMP | | | | | | | 0 | 0 | 0 | N/A | 0 | 0 | 0 | N/A |
| BMP 5 | Select a BMP | | | | | | | 0 | 0 | 0 | N/A | 0 | 0 | 0 | N/A |
| BMP 6 | Select a BMP | | | | | | | 0 | 0 | 0 | N/A | 0 | 0 | 0 | N/A |
| BMP 7 | Select a BMP | | | | | | | 0 | 0 | 0 | N/A | 0 | 0 | 0 | N/A |
| BMP 8 | Select a BMP | | | | | | | 0 | 0 | 0 | N/A | 0 | 0 | 0 | N/A |
| BMP 9 | Select a BMP | | | | | | | 0 | 0 | 0 | N/A | 0 | 0 | 0 | N/A |
| BMP 10 | 0 Select a BMP | | | | | | | 0 | 0 | 0 | N/A | 0 | 0 | 0 | N/A |
| | TOTAL | 5.00 | 1.79 | 0.00 | | | | 7,080 | | | | 806 | | 8,496 | |
| | UNTREATED AREA (acres) | 1.21 | 0.00 | | | | ı | | | | - | | - | | - |

| 7,300 | No | 6,392 | |
|-------------------------------------|------------------|--|--|
| Target Runoff Reduction Volume (cf) | Target Achieved? | Remaining Runoff Reduction Volume (cf) | |

| %0 | Remaining TSS Removal % |
|-------|----------------------------------|
| Yes! | Target Achieved? |
| %08 | % TSS Removal Achieved |
| 8,760 | Target Water Quality Volume (cf) |
| | |

Channel and Flood Protection Calculations

CURVE NUMBER CALCULATIONS

PRE-DEVELOPMENT CONDITIONS

| Basin A1- Site | | | | |
|---------------------|-----------|-----------|------------------|----|
| <u>Condition</u> | <u>CN</u> | Area (ac) | <u>CN * Area</u> | |
| Site Grassed | 61 | 4.20 | 256.20 | |
| Site Wooded | 55 | 2.80 | 154.00 | |
| TOTAL | 33 | 7.00 | 410.20 | 59 |
| TOTAL | | 7.00 | 110.20 | |
| Basin A2- to Ex. De | tention | Pond | | |
| <u>Condition</u> | <u>CN</u> | Area (ac) | CN * Area | |
| Site Wooded | 55 | 0.50 | 27.50 | |
| Offsite Impervious | 98 | 7.50 | 735.00 | |
| Offsite Grassed | 61 | 7.00 | 427.00 | |
| Offsite Wooded | 55 | 1.00 | 55.00 | |
| TOTAL | | 16.00 | 1244.50 | 78 |
| Basin A3- Bypass | | | | |
| Condition | <u>CN</u> | Area (ac) | CN * Area | |
| Offsite Impervious | 98 | 0.50 | 49.00 | |
| Offsite Grassed | 61 | 2.50 | 152.50 | |
| Offsite Wooded | 55 | 0.30 | 16.50 | |
| Adjacent Properties | 70 | 20.20 | 1414.00 | |
| TOTAL | | 23.50 | 1632.00 | 69 |
| Offsite Basin 1 | | | | |
| <u>Condition</u> | <u>CN</u> | Area (ac) | CN * Area | |
| Commercial | 80 | 6.00 | 480.00 | |
| Brook Run Park | 60 | 11.80 | 708.00 | |
| TOTAL | | 17.80 | 1188.00 | 67 |
| Offsite Basin 2 | | | | |
| Condition | <u>CN</u> | Area (ac) | CN * Area | |
| Brook Run Park | 60 | 15.50 | 930.00 | |
| TOTAL | 30 | 15.50 | 930.00 | 60 |
| | | | | |

CURVE NUMBER CALCULATIONS

POST-DEVELOPMENT CONDITIONS

| <u>Condition</u> | <u>CN</u> | <u>Area (ac)</u> | <u>CN * Area</u> | |
|------------------------|-----------|------------------|------------------|----|
| Prop. Parking | 98 | 0.34 | 33.32 | |
| Prop. Plaza | 98 | 1.17 | 114.52 | |
| Prop. Artificial Turf | 85 | 5.00 | 425.00 | |
| Grassed | 61 | 0.65 | 39.74 | |
| Future School Addition | 98 | 0.28 | 27.44 | |
| Future School Addition | 61 | 0.22 | 13.42 | |
| Offsite Impervious | 98 | 0.06 | 5.88 | |
| Offsite Grassed | 61 | 0.21 | 12.81 | |
| TOTAL | | 7.93 | 672.13 | 85 |
| | | | | |

Basin A1.2- to proposed pond

| <u>Condition</u> | <u>CN</u> | <u>Area (ac)</u> | <u>CN * Area</u> | |
|------------------|-----------|------------------|------------------|----|
| Offsite Grassed | 61 | 0.45 | 27.45 | |
| TOTAL | | 0.45 | 27.45 | 61 |

Basin A2- to Ex. Detention Pond

| <u>Condition</u> | <u>CN</u> | <u>Area (ac)</u> | <u>CN * Area</u> | |
|--------------------|-----------|------------------|------------------|----|
| Site Grassed | 61 | 0.03 | 1.83 | |
| Offsite Impervious | 98 | 7.50 | 735.00 | |
| Offsite Grassed | 61 | 5.65 | 344.65 | |
| Offsite Wooded | 55 | 0.55 | 30.25 | |
| TOTAL | | 13.73 | 1111.73 | 81 |
| | | | | |

Basin A3- Bypass

| <u>CN</u> | <u>Area (ac)</u> | <u>CN * Area</u> | |
|-----------|----------------------|--|--|
| 61 | 0.31 | 18.91 | |
| 98 | 0.50 | 49.00 | |
| 61 | 3.25 | 198.25 | |
| 55 | 0.13 | 7.15 | |
| 70 | 20.20 | 1414.00 | |
| | 24.39 | 1687.31 | 69 |
| | 61 98 61 55 | 61 0.31 98 0.50 61 3.25 55 0.13 70 20.20 | 61 0.31 18.91 98 0.50 49.00 61 3.25 198.25 55 0.13 7.15 70 20.20 1414.00 |

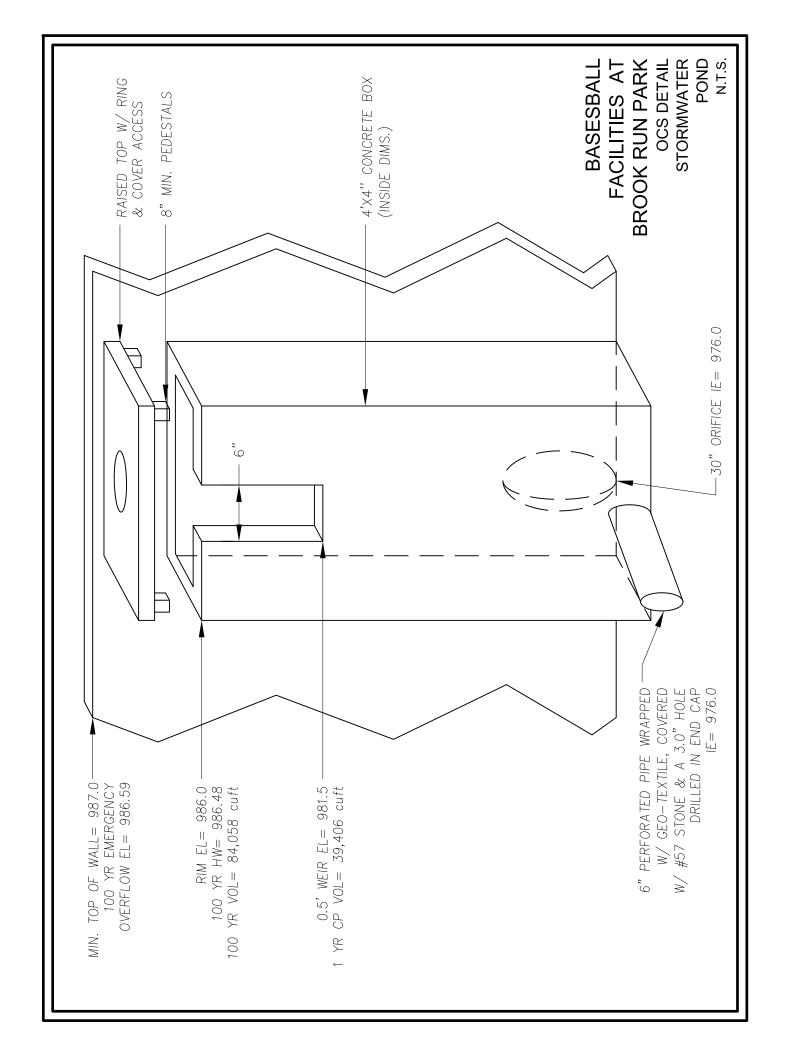
ONE YEAR CHANNEL PROTECTION CALCULATIONS

STORMWATER POND

| Area | CN | |
|-----------------|--------|--|
| 8.38 | 83 | |
| P = | 3.36 | Precipitation (in) |
| S = | 2.01 | ((1000-P)/CN)-10 |
| Q = | 1.76 | $(P - 0.2S)^2 / (P + 0.8S)$ |
| Ia= | 0.40 | .2S |
| Ia / P = | 0.12 | |
| qu= | 990 | GSWMM fig 2.1.5-6 |
| qo/qi = | 0.02 | GSWMM fig 2.2.5-1 |
| $V_{S}/V_{f} =$ | 0.65 | $.682 - 1.43(q_o/q_i) + 1.64(q_o/q_i)^2 + .804(q_o/q_i)^3$ |
| Vol Reqd = | 35,065 | (Vs/Vf) * (Q/12) * (A/43560) |
| Vol provided = | 39,406 | |
| 1-yr CP el = | 981.50 | |

The one yr channel protection orifice was calculated using the following equation: $A = (V/t) \, / \, (C \, x \, (2g(H/2)^{0.5}))$

32.2 ft/s/s C =0.6 H =5.50 ft t =86,400 sec 1 yr Channel Protection Vol = 39,406 cuft Area required = 0.0571 sf Diameter required = 3.2 in 3.0 in Diameter provided= 976.00 ft Invert el=



Hydrograph Return Period Recap Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

| Hyd. | | | | | | Peak Out | flow (cfs) | | | | Hydrograph |
|------|------------------|------------|-------|-------|------|----------|------------|--------|--------|--------|-------------------------------------|
| No. | type (origin) | Hyd(s) | 1-Yr | 2-Yr | 3-Yr | 5-Yr | 10-Yr | 25-Yr | 50-Yr | 100-Yr | description |
| 1 | SCS Runoff | | 31.28 | 44.13 | | 57.53 | 71.26 | 89.87 | 103.95 | 118.09 | Pre Basin A2- to Ex. Detention Pond |
| 2 | Reservoir | 1 | 2.812 | 3.429 | | 3.956 | 7.416 | 15.85 | 23.54 | 32.13 | Pre Ex. Pond |
| 3 | SCS Runoff | | 2.906 | 6.173 | | 10.09 | 14.44 | 20.74 | 25.76 | 31.02 | Pre Basin A1- site |
| 4 | SCS Runoff | | 20.44 | 32.72 | | 46.17 | 60.46 | 80.62 | 96.26 | 112.23 | Pre Basin A3 - bypass |
| 5 | Combine | 2, 3, 4 | 25.45 | 40.91 | | 58.01 | 76.36 | 106.07 | 133.27 | 161.43 | Pre total to Study Pt. A |
| 6 | Reservoir | 5 | 25.44 | 40.90 | | 58.02 | 76.32 | 105.91 | 132.72 | 158.40 | Pre Study Pt. A |
| 7 | SCS Runoff | | 20.45 | 33.15 | | 47.09 | 61.94 | 82.75 | 99.03 | 115.75 | Offsite Basin 1 |
| 8 | Combine | 6, 7 | 40.71 | 67.18 | | 96.31 | 127.85 | 171.26 | 205.95 | 243.65 | Pre Georgia Way S |
| 9 | SCS Runoff | | 6.011 | 12.36 | | 19.97 | 28.43 | 40.67 | 50.41 | 60.65 | Offsite Basin 2 |
| 10 | Combine | 8, 9 | 45.83 | 78.00 | | 114.53 | 153.93 | 208.88 | 253.81 | 299.36 | Pre DS Study Pt. |
| 12 | SCS Runoff | | 30.98 | 42.49 | | 54.32 | 66.33 | 82.48 | 94.64 | 106.80 | Post Basin A2 to Ex. Pond |
| 13 | Reservoir | 12 | 2.835 | 3.397 | | 3.874 | 6.232 | 13.03 | 19.74 | 26.91 | Post Ex. Pond |
| 14 | SCS Runoff | | 19.36 | 25.73 | | 32.18 | 38.66 | 47.31 | 53.79 | 60.25 | Post Basin A1.1 - to prop. pond |
| 15 | SCS Runoff | | 0.354 | 0.660 | | 1.014 | 1.399 | 1.952 | 2.388 | 2.838 | Post Basin A1.2 - to prop. pond |
| 16 | Combine | 14, 15 | 19.58 | 26.10 | | 32.73 | 39.42 | 48.43 | 55.19 | 61.94 | Post total to prop. pond |
| 17 | Reservoir | 16 | 0.534 | 1.551 | | 3.692 | 7.055 | 12.41 | 17.47 | 32.10 | Prop. pond |
| 18 | SCS Runoff | | 21.21 | 33.96 | | 47.92 | 62.75 | 83.67 | 99.91 | 116.48 | Post Basin A3 - bypass |
| 19 | Combine | 13, 17, 18 | 24.15 | 37.50 | | 51.97 | 69.21 | 99.53 | 126.62 | 160.18 | Post total to Study Pt. A |
| 20 | Reservoir | 19 | 24.16 | 37.48 | | 51.97 | 69.17 | 99.39 | 126.38 | 157.42 | Post Study Pt. A |
| 21 | Combine | 7, 20 | 39.01 | 62.49 | | 88.28 | 116.00 | 155.65 | 187.86 | 224.11 | Post Georgia Way S |
| 22 | Combine | 9, 21 | 44.04 | 73.31 | | 106.42 | 142.00 | 192.97 | 235.96 | 282.80 | Post DS Study Pt. |
| 24 | Reservoir | 16 | 0.517 | 1.552 | | 3.825 | 7.404 | 13.09 | 21.59 | 36.50 | Emergency Overflow |
| | | | | | | | | | | | |
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Proj. file: 07-11-17.gpw Monday, Jul 10, 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

| Hyd. No. | Hydrograph type (origin) | Peak flow (cfs) | Time interval (min) | Time to peak (min) | Hyd. volume (cuft) | Inflow hyd(s) | Maximum elevation (ft) | Total strge used (cuft) | Hydrograph description |
|-------------|--------------------------------|-----------------------|---------------------------|--------------------------|--------------------------|------------------|------------------------------|-------------------------------|-------------------------------------|
| 1 | SCS Runoff | 31.28 | 1 | 721 | 79,574 | | | | Pre Basin A2- to Ex. Detention Pond |
| 2 | Reservoir | 2.812 | 1 | 766 | 79,554 | 1 | 985.13 | 61,609 | Pre Ex. Pond |
| 3 | SCS Runoff | 2.906 | 1 | 724 | 10,885 | | | | Pre Basin A1- site |
| 4 | SCS Runoff | 20.44 | 1 | 727 | 75,035 | | | | Pre Basin A3 - bypass |
| 5 | Combine | 25.45 | 1 | 727 | 165,474 | 2, 3, 4 | | | Pre total to Study Pt. A |
| 6 | Reservoir | 25.44 | 1 | 727 | 165,474 | 5 | 970.49 | 173 | Pre Study Pt. A |
| 7 | SCS Runoff | 20.45 | 1 | 720 | 49,921 | | | | Offsite Basin 1 |
| 8 | Combine | 40.71 | 1 | 722 | 215,395 | 6, 7 | | | Pre Georgia Way S |
| 9 | SCS Runoff | 6.011 | 1 | 727 | 26,281 | | | | Offsite Basin 2 |
| 10 | Combine | 45.83 | 1 | 723 | 241,676 | 8, 9 | | | Pre DS Study Pt. |
| 12 | SCS Runoff | 30.98 | 1 | 721 | 78,298 | | | | Post Basin A2 to Ex. Pond |
| 13 | Reservoir | 2.835 | 1 | 762 | 78,278 | 12 | 985.18 | 62,185 | Post Ex. Pond |
| 14 | SCS Runoff | 19.36 | 1 | 723 | 55,231 | | | | Post Basin A1.1 - to prop. pond |
| 15 | SCS Runoff | 0.354 | 1 | 719 | 861 | | | | Post Basin A1.2 - to prop. pond |
| 16 | Combine | 19.58 | 1 | 723 | 56,092 | 14, 15 | | | Post total to prop. pond |
| 17 | Reservoir | 0.534 | 1 | 977 | 52,337 | 16 | 981.22 | 37,232 | Prop. pond |
| 18 | SCS Runoff | 21.21 | 1 | 727 | 77,877 | | | | Post Basin A3 - bypass |
| 19 | Combine | 24.15 | 1 | 727 | 208,491 | 13, 17, 18 | | | Post total to Study Pt. A |
| 20 | Reservoir | 24.16 | 1 | 728 | 208,487 | 19 | 970.45 | 160 | Post Study Pt. A |
| 21 | Combine | 39.01 | 1 | 722 | 258,408 | 7, 20 | | | Post Georgia Way S |
| 22 | Combine | 44.04 | 1 | 723 | 284,689 | 9, 21 | | | Post DS Study Pt. |
| 24 | Reservoir | 0.517 | 1 | 994 | 17,341 | 16 | 981.95 | 42,996 | Emergency Overflow |
| | | | | | | | | | |
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| | | 1 | 1 | l | I . | 1 | | 1 | |

07-11-17.gpw Return Period: 1 Year Monday, Jul 10, 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 1

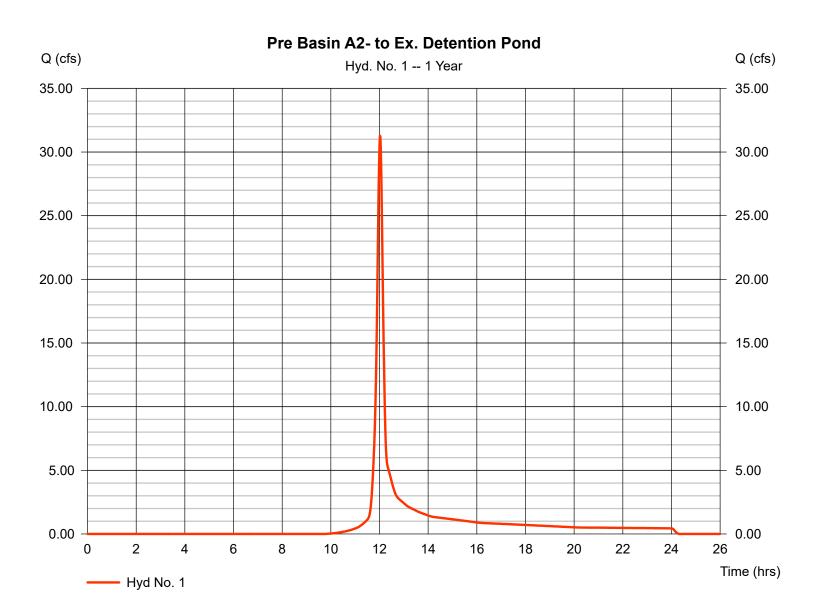
Pre Basin A2- to Ex. Detention Pond

= SCS Runoff Hydrograph type Storm frequency = 1 yrsTime interval = 1 min Drainage area = 16.000 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 3.36 inStorm duration = 24 hrs

Peak discharge = 31.28 cfs
Time to peak = 12.02 hrs
Hyd. volume = 79,574 cuft
Curve number = 78

Curve number = 78 Hydraulic length = 0 ft

Time of conc. (Tc) = 12.40 min
Distribution = Type II
Shape factor = 484



Hyd. No. 1Pre Basin A2- to Ex. Detention Pond

| <u>Description</u> | | <u>A</u> | | <u>B</u> | | <u>c</u> | | <u>Totals</u> | |
|--|-------|--|---|--|---|--|---|---------------|--|
| Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) | = | 0.150 100.0 4.08 3.50 | | 0.011 0.0 0.00 0.00 | | 0.011 0.0 0.00 0.00 | | | |
| Travel Time (min) | = | 6.94 | + | 0.00 | + | 0.00 | = | 6.94 | |
| Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s) | = | 135.00 7.00 Unpaved 4.27 | t | 1125.00 3.50 Paved 3.80 | | 0.00 0.00 Paved 0.00 | | | |
| Travel Time (min) | = | 0.53 | + | 4.93 | + | 0.00 | = | 5.46 | |
| Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft) | = = = | 0.00 0.00 0.00 0.015 0.00 0.0 | | 0.00 0.00 0.00 0.015 0.00 0.0 | | 0.00 0.00 0.00 0.015 0.00 0.0 | | | |
| Travel Time (min) | = | 0.00 | + | 0.00 | + | 0.00 | = | 0.00 | |
| Total Travel Time, Tc | | | | | | | | | |

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

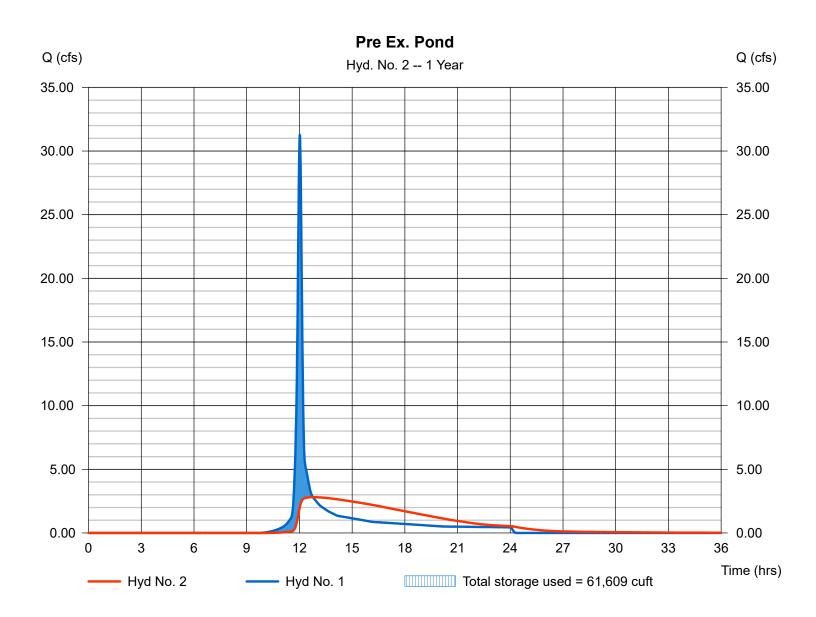
Monday, Jul 10, 2017

Hyd. No. 2

Pre Ex. Pond

Hydrograph type = Reservoir Peak discharge = 2.812 cfsStorm frequency Time to peak = 1 yrs $= 12.77 \, hrs$ Time interval = 1 min Hyd. volume = 79,554 cuft Inflow hyd. No. = 1 - Pre Basin A2- to Ex. Detention Pond Max. Elevation = 985.13 ftReservoir name = Ex. Pond Max. Storage = 61,609 cuft

Storage Indication method used. Wet pond routing start elevation = 982.00 ft.



Pond Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Pond No. 1 - Ex. Pond

Pond Data

Contours - User-defined contour areas. Conic method used for volume calculation. Begining Elevation = 978.00 ft

Stage / Storage Table

| Stage (ft) | Elevation (ft) | Contour area (sqft) | Incr. Storage (cuft) | Total storage (cuft) |
|------------|----------------|---------------------|----------------------|----------------------|
| 0.00 | 978.00 | 00 | 0 | 0 |
| 2.00 | 980.00 | 8,833 | 5,888 | 5,888 |
| 4.00 | 982.00 | 10,445 | 19,254 | 25,142 |
| 6.00 | 984.00 | 11,739 | 22,169 | 47,311 |
| 8.00 | 986.00 | 13,524 | 25,239 | 72,550 |
| 10.00 | 988.00 | 15,175 | 28,680 | 101,231 |
| 12.00 | 990.00 | 17,127 | 32,279 | 133,510 |
| 14.00 | 992.00 | 19,706 | 36,799 | 170,309 |
| 16.00 | 994.00 | 21,589 | 41,277 | 211,585 |

Culvert / Orifice Structures

Weir Structures

| | [A] | [B] | [C] | [PrfRsr] | | [A] | [B] | [C] | [D] |
|-----------------|----------|--------|------|----------|----------------|-------------|-----------|------|------|
| Rise (in) | = 42.00 | 8.00 | 0.00 | 0.00 | Crest Len (ft) | = 16.75 | 1.25 | 0.00 | 0.00 |
| Span (in) | = 42.00 | 8.00 | 0.00 | 0.00 | Crest El. (ft) | = 992.30 | 988.00 | 0.00 | 0.00 |
| No. Barrels | = 1 | 1 | 0 | 0 | Weir Coeff. | = 3.33 | 3.33 | 3.33 | 3.33 |
| Invert El. (ft) | = 976.50 | 982.00 | 0.00 | 0.00 | Weir Type | = Riser | Rect | | |
| Length (ft) | = 0.00 | 0.00 | 0.00 | 0.00 | Multi-Stage | = Yes | Yes | No | No |
| Slope (%) | = 0.00 | 0.00 | 0.00 | n/a | | | | | |
| N-Value | = .013 | .013 | .013 | n/a | | | | | |
| Orifice Coeff. | = 0.60 | 0.60 | 0.60 | 0.60 | Exfil.(in/hr) | = 0.000 (by | Wet area) | | |
| Multi-Stage | = n/a | Yes | No | No | TW Elev. (ft) | = 0.00 | | | |

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Stage / Storage / Discharge Table

| Stage ft | Storage cuft | Elevation ft | Clv A cfs | Clv B cfs | Clv C cfs | PrfRsr cfs | Wr A cfs | Wr B cfs | Wr C cfs | Wr D cfs | Exfil cfs | User cfs | Total cfs |
|-------------|--------------|-----------------|--------------|--------------|--------------|---------------|-------------|-------------|-------------|-------------|--------------|-------------|--------------|
| 0.00 | 0 | 978.00 | 0.00 | 0.00 | | | 0.00 | 0.00 | | | | | 0.000 |
| 2.00 | 5,888 | 980.00 | 16.47 ic | 0.00 | | | 0.00 | 0.00 | | | | | 0.000 |
| 4.00 | 25,142 | 982.00 | 16.47 ic | 0.00 | | | 0.00 | 0.00 | | | | | 0.000 |
| 6.00 | 47,311 | 984.00 | 16.47 ic | 2.17 ic | | | 0.00 | 0.00 | | | | | 2.170 |
| 8.00 | 72,550 | 986.00 | 16.47 ic | 3.22 ic | | | 0.00 | 0.00 | | | | | 3.218 |
| 10.00 | 101,231 | 988.00 | 16.47 ic | 4.00 ic | | | 0.00 | 0.00 | | | | | 4.000 |
| 12.00 | 133,510 | 990.00 | 16.47 ic | 4.65 ic | | | 0.00 | 11.77 | | | | | 16.43 |
| 14.00 | 170,309 | 992.00 | 38.64 ic | 5.22 ic | | | 0.00 | 33.30 | | | | | 38.52 |
| 16.00 | 211,585 | 994.00 | 171.52 ic | 2.40 ic | | | 123.63 | 45.49 s | | | | | 171.52 |

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

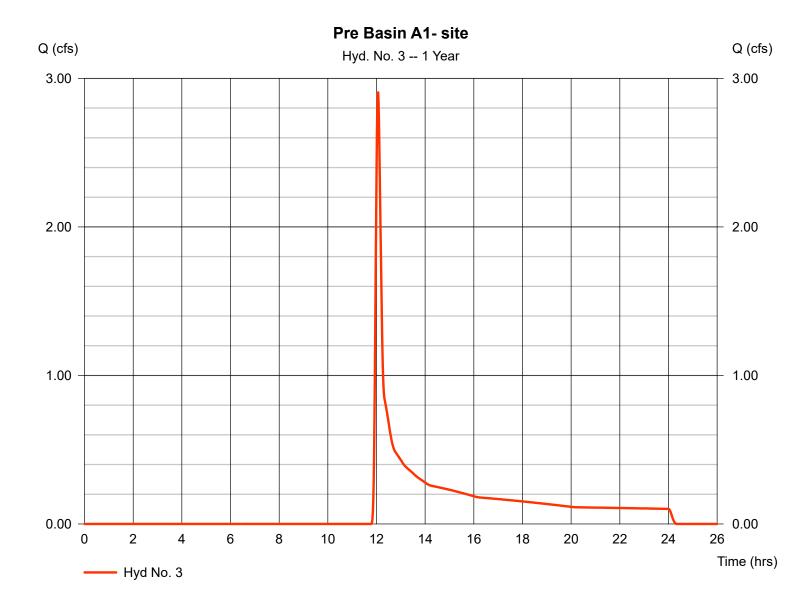
Hyd. No. 3

Pre Basin A1- site

Hydrograph type = SCS Runoff Storm frequency = 1 yrsTime interval = 1 min Drainage area = 7.000 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 3.36 inStorm duration = 24 hrs

Peak discharge = 2.906 cfs
Time to peak = 12.07 hrs
Hyd. volume = 10,885 cuft
Curve number = 59
Hydraulic length = 0 ft

Time of conc. (Tc) = 12.80 min
Distribution = Type II
Shape factor = 484



Hyd. No. 3

Pre Basin A1- site

| <u>Description</u> | | <u>A</u> | | <u>B</u> | | <u>C</u> | | <u>Totals</u> | | |
|--|-------|--|---|--|---|--|---|---------------|--|--|
| Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) | = | 0.150 100.0 4.08 5.00 | | 0.011 0.0 0.00 0.00 | | 0.011 0.0 0.00 0.00 | | | | |
| Travel Time (min) | = | 6.01 | + | 0.00 | + | 0.00 | = | 6.01 | | |
| Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s) | = = = | 630.00 3.30 Unpaved 2.93 | | 700.00 3.30 Paved 3.69 | | 0.00 0.00 Paved 0.00 | | | | |
| Travel Time (min) | = | 3.58 | + | 3.16 | + | 0.00 | = | 6.74 | | |
| Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft) | = = = | 0.00 0.00 0.00 0.015 0.00 0.0 | | 0.00 0.00 0.00 0.015 0.00 0.0 | | 0.00 0.00 0.00 0.015 0.00 0.0 | | | | |
| Travel Time (min) | = | 0.00 | + | 0.00 | + | 0.00 | = | 0.00 | | |
| Total Travel Time, Tc | | | | | | | | | | |

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Monday, Jul 10, 2017

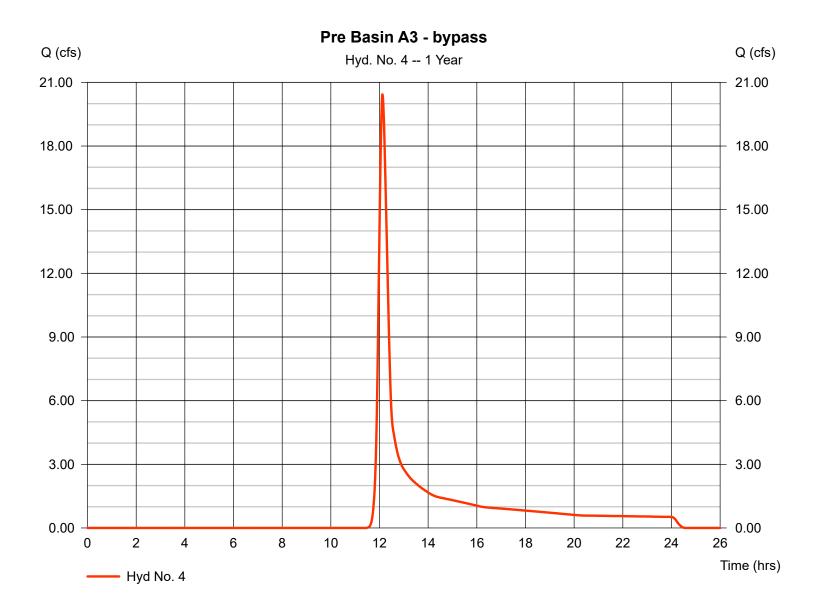
Hyd. No. 4

Pre Basin A3 - bypass

Hydrograph type = SCS Runoff Storm frequency = 1 yrsTime interval = 1 min Drainage area = 23.500 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 3.36 inStorm duration = 24 hrs

Peak discharge = 20.44 cfs
Time to peak = 12.12 hrs
Hyd. volume = 75,035 cuft
Curve number = 69

Hydraulic length = 0 ft
Time of conc. (Tc) = 21.80 min
Distribution = Type II
Shape factor = 484



Hyd. No. 4Pre Basin A3 - bypass

| <u>Description</u> | <u>A</u> | | <u>B</u> | | <u>C</u> | | <u>Totals</u> |
|--|--|---|--|---|--|---|---------------|
| Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) | = 0.150 = 100.0 = 4.08 = 2.00 | | 0.011 0.0 0.00 0.00 | | 0.011 0.0 0.00 0.00 | | |
| Travel Time (min) | = 8.68 | + | 0.00 | + | 0.00 | = | 8.68 |
| Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s) | = 1265.00 = 2.00 = Paved = 2.87 | | 1285.00 3.30 Paved 3.69 |) | 0.00 0.00 Paved 0.00 | | |
| Travel Time (min) | = 7.33 | + | 5.80 | + | 0.00 | = | 13.13 |
| Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft) | = 0.00 = 0.00 = 0.00 = 0.015 = 0.00 = 0.0 | | 0.00 0.00 0.00 0.015 0.00 0.0 | | 0.00 0.00 0.00 0.015 0.00 0.0 | | |
| Travel Time (min) | = 0.00 | + | 0.00 | + | 0.00 | = | 0.00 |
| Total Travel Time, Tc | | | | | | | 21.80 mir |

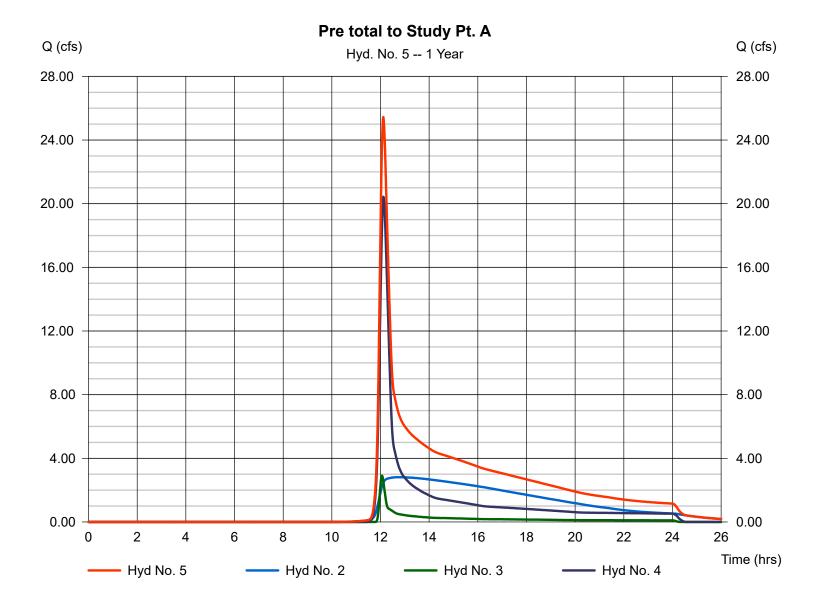
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Monday, Jul 10, 2017

Hyd. No. 5

Pre total to Study Pt. A

Hydrograph type = Combine Storm frequency = 1 yrs Time interval = 1 min Inflow hyds. = 2, 3, 4 Peak discharge = 25.45 cfs Time to peak = 12.12 hrs Hyd. volume = 165,474 cuft Contrib. drain. area = 30.500 ac



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

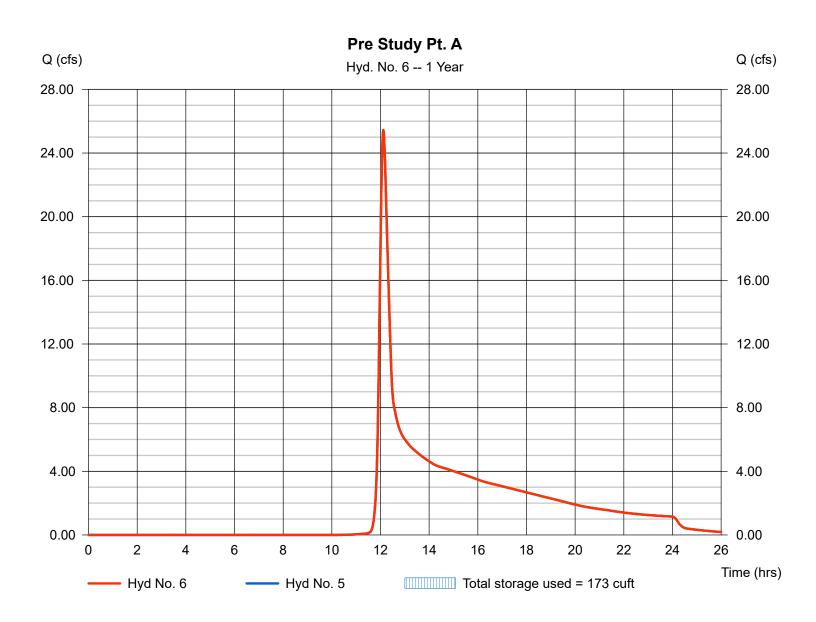
Monday, Jul 10, 2017

Hyd. No. 6

Pre Study Pt. A

Hydrograph type = Reservoir Peak discharge = 25.44 cfsStorm frequency Time to peak = 1 yrs $= 12.12 \, hrs$ Time interval = 1 min Hyd. volume = 165,474 cuft Inflow hyd. No. = 5 - Pre total to Study Pt. A Max. Elevation = 970.49 ftReservoir name = Ex. DS Culvert at Barclay Max. Storage = 173 cuft

Storage Indication method used.



Pond Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Pond No. 4 - Ex. DS Culvert at Barclay

Pond Data

Contours - User-defined contour areas. Conic method used for volume calculation. Begining Elevation = 968.76 ft

Stage / Storage Table

| Stage (ft) | ge (ft) Elevation (ft) Contour a | | Incr. Storage (cuft) | Total storage (cuft) |
|------------|----------------------------------|--------|----------------------|----------------------|
| 0.00 | 968.76 | 00 | 0 | 0 |
| 1.24 | 970.00 | 108 | 45 | 45 |
| 3.24 | 972.00 | 450 | 519 | 564 |
| 5.24 | 974.00 | 980 | 1,396 | 1,959 |
| 7.24 | 976.00 | 3,970 | 4,615 | 6,574 |
| 9.24 | 978.00 | 9,530 | 13,099 | 19,673 |
| 11.24 | 980.00 | 13,858 | 23,251 | 42,924 |
| 13.24 | 982.00 | 18,108 | 31,868 | 74,793 |

Culvert / Orifice Structures Weir Structures [A] [B] [C] [PrfRsr] [A] [B] [C] [D] Rise (in) = 54.00 0.00 0.00 0.00 Crest Len (ft) = 0.00 0.00 0.00 0.00 Span (in) = 54.00 0.00 0.00 0.00 Crest El. (ft) = 0.00 0.00 0.00 0.00 No. Barrels = 1 0 0 Weir Coeff. = 3.330 3.33 3.33 3.33 Invert El. (ft) = 968.76 0.00 0.00 0.00 Weir Type = ---Length (ft) = 0.000.00 0.00 0.00 Multi-Stage = No No No No Slope (%) = 0.000.00 0.00 n/a = .013 .013 .013 n/a N-Value Orifice Coeff. = 0.600.60 0.60 0.60 Exfil.(in/hr) = 0.000 (by Wet area) Multi-Stage = n/a No TW Elev. (ft) = 0.00

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Stage / Storage / Discharge Table

| Stage ft | Storage cuft | Elevation ft | Clv A cfs | Clv B cfs | CIv C cfs | PrfRsr cfs | Wr A cfs | Wr B cfs | Wr C cfs | Wr D cfs | Exfil cfs | User cfs | Total cfs |
|-------------|-----------------|-----------------|--------------|--------------|--------------|---------------|-------------|-------------|-------------|-------------|--------------|-------------|--------------|
| 0.00 | 0 | 968.76 | 0.00 | | | | | | | | | | 0.000 |
| 1.24 | 45 | 970.00 | 13.53 ic | | | | | | | | | | 13.53 |
| 3.24 | 564 | 972.00 | 75.20 ic | | | | | | | | | | 75.20 |
| 5.24 | 1,959 | 974.00 | 132.40 ic | | | | | | | | | | 132.40 |
| 7.24 | 6,574 | 976.00 | 171.04 ic | | | | | | | | | | 171.04 |
| 9.24 | 19,673 | 978.00 | 202.44 ic | | | | | | | | | | 202.44 |
| 11.24 | 42,924 | 980.00 | 229.58 ic | | | | | | | | | | 229.58 |
| 13.24 | 74,793 | 982.00 | 253.84 ic | | | | | | | | | | 253.84 |

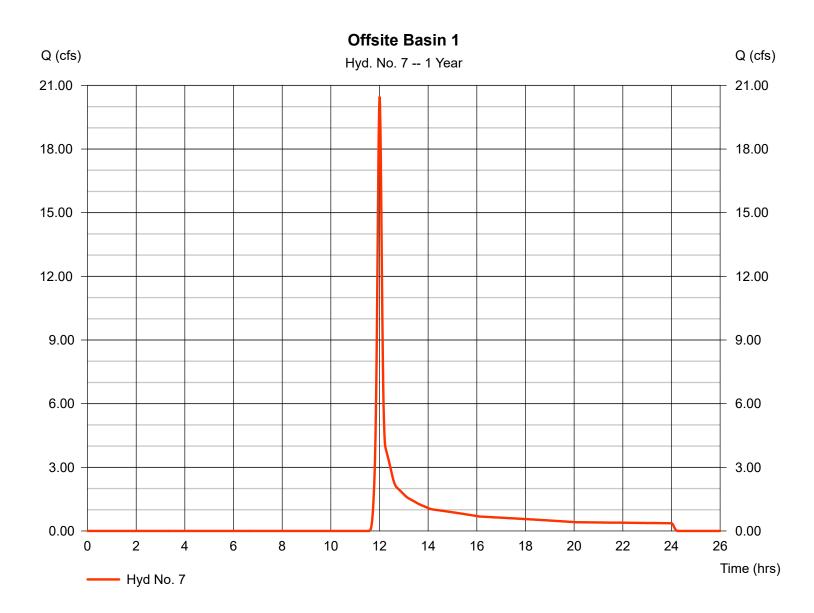
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 7

Offsite Basin 1

Hydrograph type = SCS Runoff Peak discharge = 20.45 cfsStorm frequency Time to peak = 12.00 hrs= 1 yrsTime interval = 1 min Hyd. volume = 49,921 cuft Drainage area = 17.800 acCurve number = 67 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.80 \, \text{min}$ Distribution Total precip. = 3.36 in= Type II Storm duration = 24 hrs Shape factor = 484



Hyd. No. 7

Offsite Basin 1

| <u>Description</u> | <u>A</u> | <u>A</u> | | <u>B</u> | | | <u>Totals</u> | | |
|--|--|----------|--|----------|--|---|---------------|--|--|
| Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) | = 0.150 = 100.0 = 4.08 = 16.00 | | 0.011 0.0 0.00 0.00 | | 0.011 0.0 0.00 0.00 | | | | |
| Travel Time (min) | = 3.78 | + | 0.00 | + | 0.00 | = | 3.78 | | |
| Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s) | = 980.00 = 4.00 = Unpaved = 3.23 | | 0.00 0.00 Paved 0.00 | | 0.00 0.00 Paved 0.00 | | | | |
| Travel Time (min) | = 5.06 | + | 0.00 | + | 0.00 | = | 5.06 | | |
| Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft) | = 0.00 = 0.00 = 0.00 = 0.015 = 0.00 = 0.0 | | 0.00 0.00 0.00 0.015 0.00 0.0 | | 0.00 0.00 0.00 0.015 0.00 0.0 | | | | |
| Travel Time (min) | = 0.00 | + | 0.00 | + | 0.00 | = | 0.00 | | |
| Total Travel Time, Tc | | | | | | | | | |

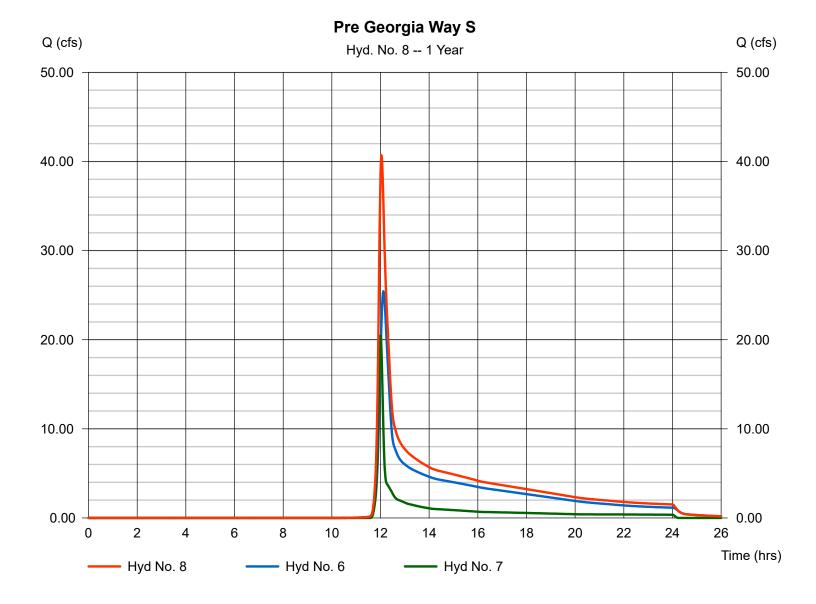
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 8

Pre Georgia Way S

Hydrograph type = Combine Storm frequency = 1 yrs Time interval = 1 min Inflow hyds. = 6, 7 Peak discharge = 40.71 cfs Time to peak = 12.03 hrs Hyd. volume = 215,395 cuft Contrib. drain. area = 17.800 ac



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 9

Offsite Basin 2

Storm duration

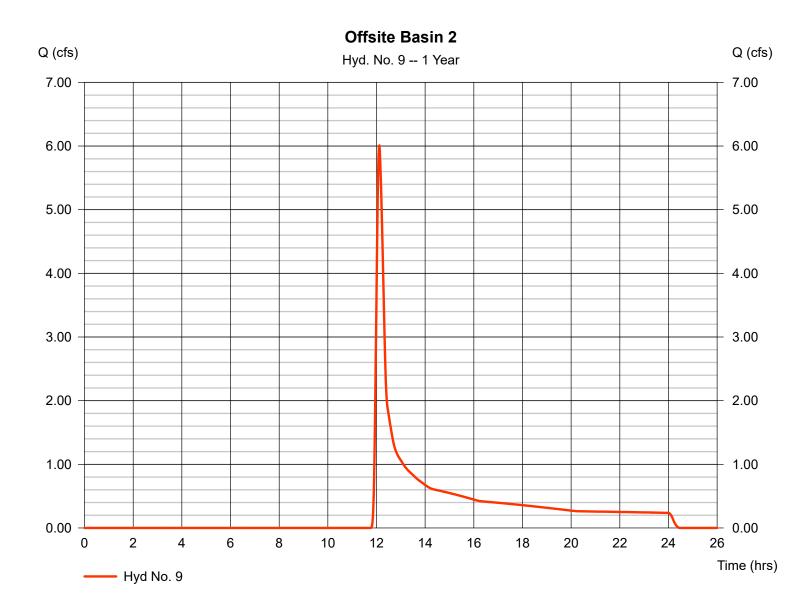
Hydrograph type = SCS Runoff
Storm frequency = 1 yrs
Time interval = 1 min
Drainage area = 15.500 ac
Basin Slope = 0.0 %
Tc method = TR55
Total precip. = 3.36 in

= 24 hrs

Peak discharge = 6.011 cfs
Time to peak = 12.12 hrs
Hyd. volume = 26,281 cuft
Curve number = 60

Curve number = 60 Hydraulic length = 0 ft Time of conc. (Tc) = 17.30 min

Distribution = Type II Shape factor = 484



Hyd. No. 9

Offsite Basin 2

| <u>Description</u> | <u>A</u> | <u>B</u> | <u>C</u> | <u>Totals</u> | | | | | |
|--|---|--|--|---------------|--|--|--|--|--|
| Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) | = 0.150 = 100.0 = 4.08 = 2.00 | 0.011 0.0 0.00 0.00 | 0.011 0.0 0.00 0.00 | | | | | | |
| Travel Time (min) | = 8.68 | + 0.00 | + 0.00 | = 8.68 | | | | | |
| Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s) Travel Time (min) | = 1445.00 = 3.00 = Unpaved = 2.79 = 8.62 | 0.00 | 0.00 0.00 Paved 0.00 | = 8.62 | | | | | |
| , , | - 0.02 | + 0.00 | T 0.00 | - 0.02 | | | | | |
| Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft) | = 0.00 = 0.00 = 0.00 = 0.015 = 0.00 = 0.0 | 0.00 0.00 0.00 0.015 0.00 0.0 | 0.00 0.00 0.00 0.015 0.00 0.0 | | | | | | |
| Travel Time (min) | = 0.00 | + 0.00 | + 0.00 | = 0.00 | | | | | |
| Total Travel Time, Tc | | | | | | | | | |

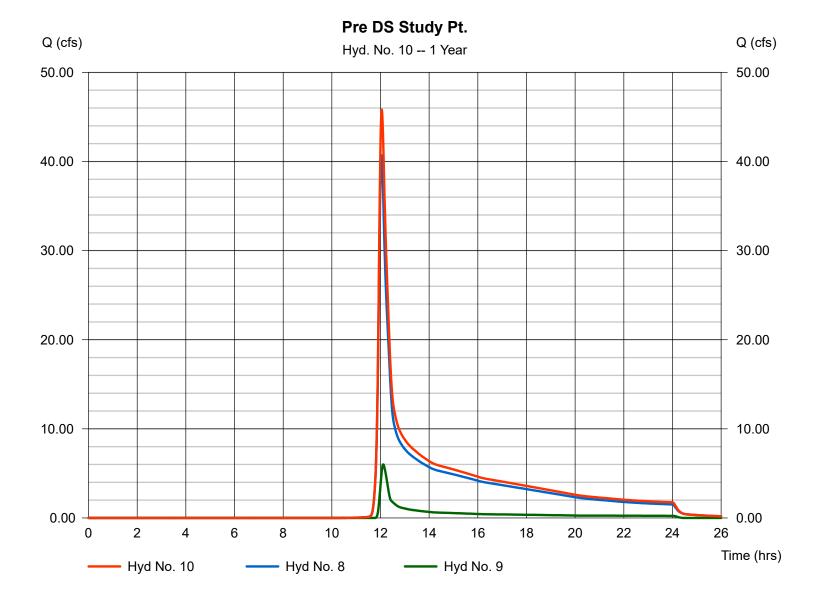
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Monday, Jul 10, 2017

Hyd. No. 10

Pre DS Study Pt.

Hydrograph type = Combine Storm frequency = 1 yrs Time interval = 1 min Inflow hyds. = 8, 9 Peak discharge = 45.83 cfs Time to peak = 12.05 hrs Hyd. volume = 241,676 cuft Contrib. drain. area = 15.500 ac



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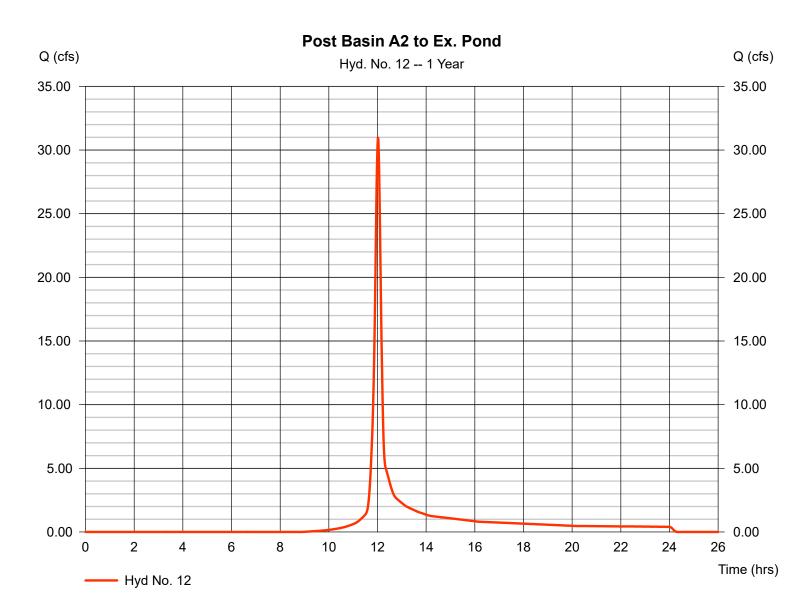
Hyd. No. 12

Post Basin A2 to Ex. Pond

Hydrograph type = SCS Runoff Storm frequency = 1 yrsTime interval = 1 min Drainage area = 13.730 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 3.36 inStorm duration = 24 hrs

Peak discharge = 30.98 cfs
Time to peak = 12.02 hrs
Hyd. volume = 78,298 cuft
Curve number = 81

Hydraulic length = 0 ft
Time of conc. (Tc) = 12.40 min
Distribution = Type II
Shape factor = 484



Hyd. No. 12Post Basin A2 to Ex. Pond

| <u>Description</u> | <u>A</u> | | <u>B</u> | | <u>C</u> | | <u>Totals</u> | | |
|--|--|---|--|---|--|---|---------------|--|--|
| Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) | = 0.150 = 100.0 = 4.08 = 3.50 | | 0.011 0.0 0.00 0.00 | | 0.011 0.0 0.00 0.00 | | | | |
| Travel Time (min) | = 6.94 | + | 0.00 | + | 0.00 | = | 6.94 | | |
| Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s) | = 135.00 = 7.00 = Unpaved = 4.27 | d | 1125.00 3.50 Paved 3.80 | | 0.00 0.00 Paved 0.00 | | | | |
| Travel Time (min) | = 0.53 | + | 4.93 | + | 0.00 | = | 5.46 | | |
| Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft) | = 0.00 = 0.00 = 0.00 = 0.015 = 0.00 = 0.0 | | 0.00 0.00 0.00 0.015 0.00 0.0 | | 0.00 0.00 0.00 0.015 0.00 0.0 | | | | |
| Travel Time (min) | = 0.00 | + | 0.00 | + | 0.00 | = | 0.00 | | |
| Total Travel Time, Tc | | | | | | | | | |

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

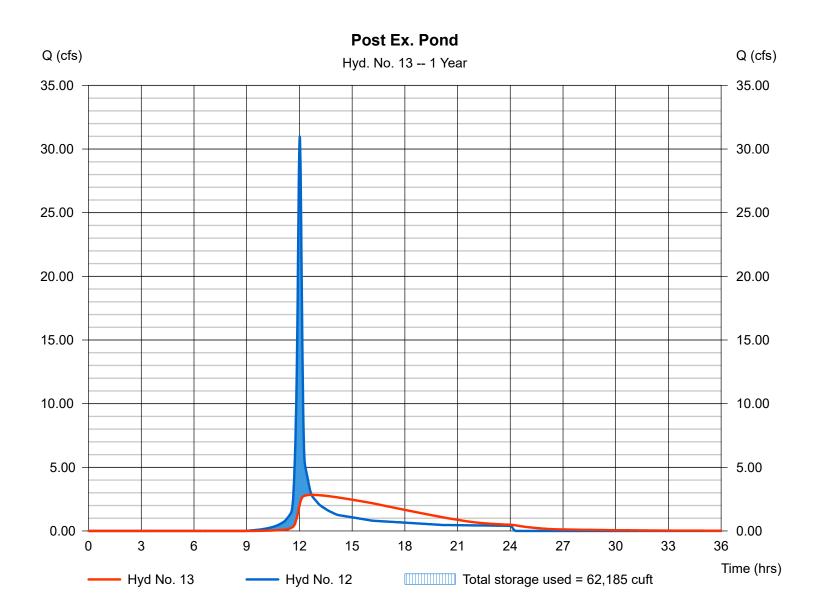
Monday, Jul 10, 2017

Hyd. No. 13

Post Ex. Pond

Hydrograph type = Reservoir Peak discharge = 2.835 cfsStorm frequency Time to peak = 1 yrs $= 12.70 \, hrs$ Time interval = 1 min Hyd. volume = 78,278 cuft Inflow hyd. No. = 12 - Post Basin A2 to Ex. Pond Max. Elevation = 985.18 ft Reservoir name = Ex. Pond Max. Storage = 62,185 cuft

Storage Indication method used. Wet pond routing start elevation = 982.00 ft.



Pond Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

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Pond No. 1 - Ex. Pond

Pond Data

Contours - User-defined contour areas. Conic method used for volume calculation. Begining Elevation = 978.00 ft

Stage / Storage Table

| Stage (ft) Elevation (ft) | | Contour area (sqft) | Incr. Storage (cuft) | Total storage (cuft) | | |
|---------------------------|--------|---------------------|----------------------|----------------------|--|--|
| 0.00 | 978.00 | 00 | 0 | 0 | | |
| 2.00 | 980.00 | 8,833 | 5,888 | 5,888 | | |
| 4.00 | 982.00 | 10,445 | 19,254 | 25,142 | | |
| 6.00 | 984.00 | 11,739 | 22,169 | 47,311 | | |
| 8.00 | 986.00 | 13,524 | 25,239 | 72,550 | | |
| 10.00 | 988.00 | 15,175 | 28,680 | 101,231 | | |
| 12.00 | 990.00 | 17,127 | 32,279 | 133,510 | | |
| 14.00 | 992.00 | 19,706 | 36,799 | 170,309 | | |
| 16.00 | 994.00 | 21,589 | 41,277 | 211,585 | | |

Culvert / Orifice Structures

Weir Structures

| | [A] | [B] | [C] | [PrfRsr] | | [A] | [B] | [C] | [D] |
|-----------------|----------|--------|------|----------|----------------|-------------|-----------|------|------|
| Rise (in) | = 42.00 | 8.00 | 0.00 | 0.00 | Crest Len (ft) | = 16.75 | 1.25 | 0.00 | 0.00 |
| Span (in) | = 42.00 | 8.00 | 0.00 | 0.00 | Crest El. (ft) | = 992.30 | 988.00 | 0.00 | 0.00 |
| No. Barrels | = 1 | 1 | 0 | 0 | Weir Coeff. | = 3.33 | 3.33 | 3.33 | 3.33 |
| Invert El. (ft) | = 976.50 | 982.00 | 0.00 | 0.00 | Weir Type | = Riser | Rect | | |
| Length (ft) | = 0.00 | 0.00 | 0.00 | 0.00 | Multi-Stage | = Yes | Yes | No | No |
| Slope (%) | = 0.00 | 0.00 | 0.00 | n/a | | | | | |
| N-Value | = .013 | .013 | .013 | n/a | | | | | |
| Orifice Coeff. | = 0.60 | 0.60 | 0.60 | 0.60 | Exfil.(in/hr) | = 0.000 (by | Wet area) | | |
| Multi-Stage | = n/a | Yes | No | No | TW Elev. (ft) | = 0.00 | | | |

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Stage / Storage / Discharge Table

| Stage ft | Storage cuft | Elevation ft | Clv A cfs | Clv B cfs | Clv C cfs | PrfRsr cfs | Wr A cfs | Wr B cfs | Wr C cfs | Wr D cfs | Exfil cfs | User cfs | Total cfs |
|-------------|--------------|-----------------|--------------|--------------|--------------|---------------|-------------|-------------|-------------|-------------|--------------|-------------|--------------|
| 0.00 | 0 | 978.00 | 0.00 | 0.00 | | | 0.00 | 0.00 | | | | | 0.000 |
| 2.00 | 5,888 | 980.00 | 16.47 ic | 0.00 | | | 0.00 | 0.00 | | | | | 0.000 |
| 4.00 | 25,142 | 982.00 | 16.47 ic | 0.00 | | | 0.00 | 0.00 | | | | | 0.000 |
| 6.00 | 47,311 | 984.00 | 16.47 ic | 2.17 ic | | | 0.00 | 0.00 | | | | | 2.170 |
| 8.00 | 72,550 | 986.00 | 16.47 ic | 3.22 ic | | | 0.00 | 0.00 | | | | | 3.218 |
| 10.00 | 101,231 | 988.00 | 16.47 ic | 4.00 ic | | | 0.00 | 0.00 | | | | | 4.000 |
| 12.00 | 133,510 | 990.00 | 16.47 ic | 4.65 ic | | | 0.00 | 11.77 | | | | | 16.43 |
| 14.00 | 170,309 | 992.00 | 38.64 ic | 5.22 ic | | | 0.00 | 33.30 | | | | | 38.52 |
| 16.00 | 211,585 | 994.00 | 171.52 ic | 2.40 ic | | | 123.63 | 45.49 s | | | | | 171.52 |

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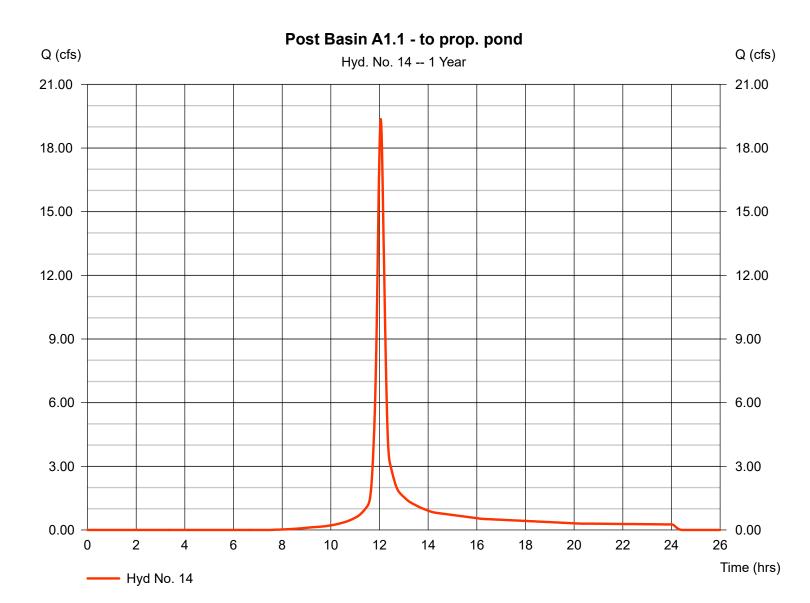
Hyd. No. 14

Post Basin A1.1 - to prop. pond

Hydrograph type = SCS Runoff Storm frequency = 1 yrsTime interval = 1 min Drainage area = 7.930 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 3.36 inStorm duration = 24 hrs

Peak discharge = 19.36 cfs
Time to peak = 12.05 hrs
Hyd. volume = 55,231 cuft
Curve number = 85
Hydraulic length = 0 ft

Time of conc. (Tc) = 16.30 min
Distribution = Type II
Shape factor = 484



Hyd. No. 14Post Basin A1.1 - to prop. pond

| <u>Description</u> | <u>A</u> | | <u>B</u> | | <u>C</u> | | <u>Totals</u> |
|--|--|---|--|---|--|---|---------------|
| Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) | = 0.011 = 100.0 = 4.08 = 1.00 | | 0.011 0.0 0.00 0.00 | | 0.011 0.0 0.00 0.00 | | |
| Travel Time (min) | = 1.42 | + | 0.00 | + | 0.00 | = | 1.42 |
| Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s) | = 1815.00 = 1.00 = Paved = 2.03 | | 0.00 0.00 Paved 0.00 | | 0.00 0.00 Paved 0.00 | | |
| Travel Time (min) | = 14.88 | + | 0.00 | + | 0.00 | = | 14.88 |
| Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft) | = 0.00 = 0.00 = 0.015 = 0.00 = 0.0 | | 0.00 0.00 0.00 0.015 0.00 0.0 | | 0.00 0.00 0.00 0.015 0.00 0.0 | | |
| Travel Time (min) | = 0.00 | + | 0.00 | + | 0.00 | = | 0.00 |
| Total Travel Time, Tc | | | | | | | 16.30 min |

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Time (hrs)

= 0.354 cfs

 $= 11.98 \, hrs$

= 861 cuft

 $= 5.00 \, \text{min}$

= Type II

= 61

= 0 ft

= 484

Hyd. No. 15

Post Basin A1.2 - to prop. pond

Hyd No. 15

Hydrograph type = SCS Runoff Peak discharge Storm frequency Time to peak = 1 yrsTime interval = 1 min Hyd. volume Drainage area = 0.450 acCurve number Basin Slope = 0.0 % Hydraulic length Tc method = USER Time of conc. (Tc) Distribution Total precip. = 3.36 inStorm duration = 24 hrs Shape factor

Post Basin A1.2 - to prop. pond Q (cfs) Q (cfs) Hyd. No. 15 -- 1 Year 0.50 0.50 0.45 0.45 0.40 0.40 0.35 0.35 0.30 0.30 0.25 0.25 0.20 0.20 0.15 0.15 0.10 0.10 0.05 0.05 0.00 0.00 2 6 8 10 14 16 18 20 22 24 26 12

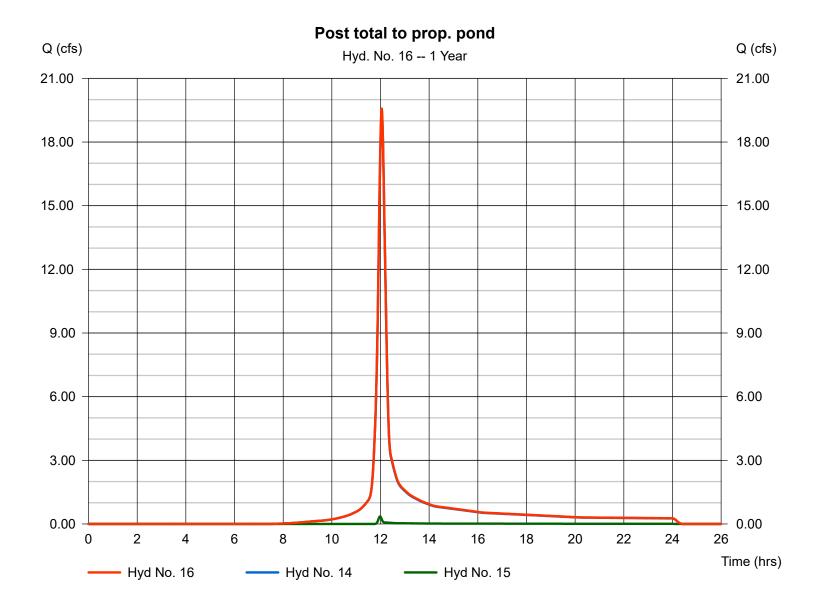
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Monday, Jul 10, 2017

Hyd. No. 16

Post total to prop. pond

Hydrograph type = Combine Storm frequency = 1 yrs Time interval = 1 min Inflow hyds. = 14, 15 Peak discharge = 19.58 cfs Time to peak = 12.05 hrs Hyd. volume = 56,092 cuft Contrib. drain. area = 8.380 ac



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

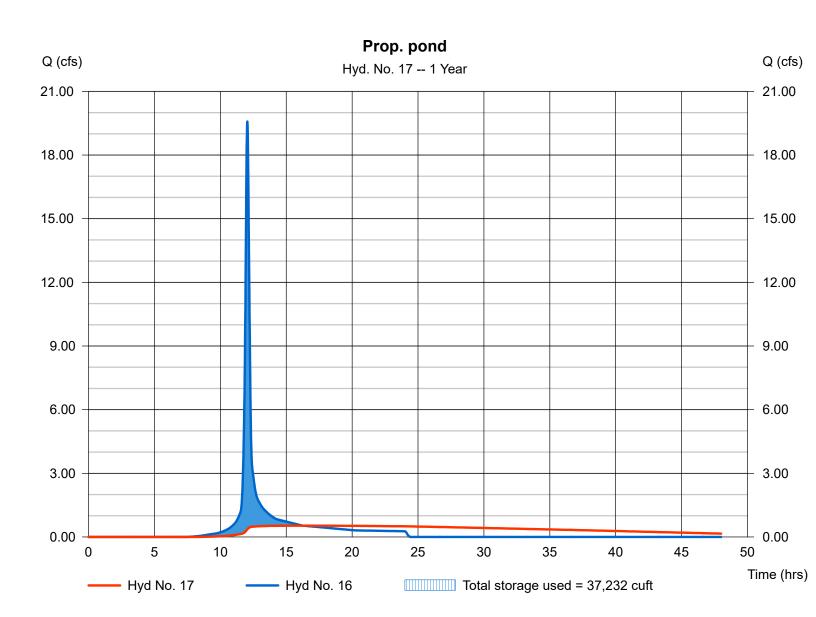
Monday, Jul 10, 2017

Hyd. No. 17

Prop. pond

Hydrograph type = Reservoir Peak discharge = 0.534 cfsStorm frequency Time to peak = 1 yrs $= 16.28 \, hrs$ Time interval = 1 min Hyd. volume = 52,337 cuft Max. Elevation Inflow hyd. No. = 16 - Post total to prop. pond = 981.22 ftReservoir name = Stormwater Pond Max. Storage = 37,232 cuft

Storage Indication method used.



Pond Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

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Pond No. 14 - Stormwater Pond

Pond Data

Contours - User-defined contour areas. Conic method used for volume calculation. Begining Elevation = 976.00 ft

Stage / Storage Table

| Stage (ft) | Elevation (ft) | Contour area (sqft) | Incr. Storage (cuft) | Total storage (cuft) |
|------------|----------------|---------------------|----------------------|----------------------|
| 0.00 | 976.00 | 6,240 | 0 | 0 |
| 2.00 | 978.00 | 6,887 | 13,120 | 13,120 |
| 4.00 | 980.00 | 7,555 | 14,435 | 27,556 |
| 6.00 | 982.00 | 8,252 | 15,800 | 43,356 |
| 8.00 | 984.00 | 8,968 | 17,213 | 60,569 |
| 10.00 | 986.00 | 9,708 | 18,669 | 79,239 |
| 12.00 | 988.00 | 10,475 | 20,176 | 99,415 |

Culvert / Orifice Structures

Weir Structures

| | [A] | [B] | [C] | [PrfRsr] | | [A] | [B] | [C] | [D] |
|-----------------|----------|--------|------|----------|----------------|-------------|-----------|------|------|
| Rise (in) | = 30.00 | 3.00 | 0.00 | 0.00 | Crest Len (ft) | = 11.50 | 0.50 | 0.00 | 0.00 |
| Span (in) | = 30.00 | 3.00 | 0.00 | 0.00 | Crest El. (ft) | = 986.00 | 981.50 | 0.00 | 0.00 |
| No. Barrels | = 1 | 1 | 0 | 0 | Weir Coeff. | = 3.33 | 3.33 | 3.33 | 3.33 |
| Invert El. (ft) | = 976.00 | 976.00 | 0.00 | 0.00 | Weir Type | = Riser | Rect | | |
| Length (ft) | = 0.00 | 0.00 | 0.00 | 0.00 | Multi-Stage | = Yes | Yes | No | No |
| Slope (%) | = 0.00 | 0.00 | 0.00 | n/a | | | | | |
| N-Value | = .013 | .013 | .013 | n/a | | | | | |
| Orifice Coeff. | = 0.60 | 0.60 | 0.60 | 0.60 | Exfil.(in/hr) | = 0.000 (by | Wet area) | | |
| Multi-Stage | = n/a | No | No | No | TW Elev. (ft) | = 0.00 | | | |

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Stage / Storage / Discharge Table

| Stage ft | Storage cuft | Elevation ft | CIv A cfs | Clv B cfs | CIv C cfs | PrfRsr cfs | Wr A cfs | Wr B cfs | Wr C cfs | Wr D cfs | Exfil cfs | User cfs | Total cfs |
|-------------|--------------|-----------------|--------------|--------------|--------------|---------------|-------------|-------------|-------------|-------------|--------------|-------------|--------------|
| 0.00 | 0 | 976.00 | 0.00 | 0.00 | | | 0.00 | 0.00 | | | | | 0.000 |
| 2.00 | 13,120 | 978.00 | 0.00 | 0.32 ic | | | 0.00 | 0.00 | | | | | 0.324 |
| 4.00 | 27,556 | 980.00 | 0.00 | 0.47 ic | | | 0.00 | 0.00 | | | | | 0.465 |
| 6.00 | 43,356 | 982.00 | 0.61 ic | 0.57 ic | | | 0.00 | 0.59 | | | | | 1.161 |
| 8.00 | 60,569 | 984.00 | 6.59 ic | 0.66 ic | | | 0.00 | 6.58 | | | | | 7.245 |
| 10.00 | 79,239 | 986.00 | 15.89 ic | 0.74 ic | | | 0.00 | 15.89 | | | | | 16.64 |
| 12.00 | 99,415 | 988.00 | 76.11 ic | 0.81 ic | | | 65.38 s | 10.72 s | | | | | 76.92 |

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Hyd. No. 18

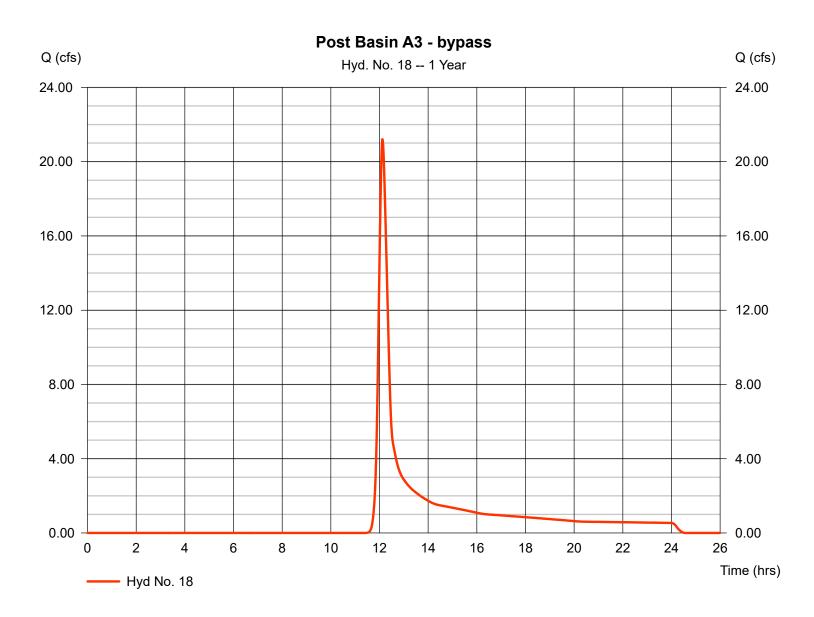
Post Basin A3 - bypass

Hydrograph type = SCS Runoff Storm frequency = 1 yrsTime interval = 1 min Drainage area = 24.390 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 3.36 inStorm duration = 24 hrs

Peak discharge = 21.21 cfs
Time to peak = 12.12 hrs
Hyd. volume = 77,877 cuft
Curve number = 69

Hydraulic length = 0 ft
Time of conc. (Tc) = 21.80 min

Distribution = Type II Shape factor = 484



Hyd. No. 18

Post Basin A3 - bypass

| <u>Description</u> | <u>A</u> | | <u>B</u> | | <u>C</u> | | <u>Totals</u> |
|--|--|---|--|---|--|---|---------------|
| Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) | = 0.150 = 100.0 = 4.08 = 2.00 | | 0.011 0.0 0.00 0.00 | | 0.011 0.0 0.00 0.00 | | |
| Travel Time (min) | = 8.68 | + | 0.00 | + | 0.00 | = | 8.68 |
| Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s) | = 1265.00 = 2.00 = Paved = 2.87 | | 1285.00 3.30 Paved 3.69 | | 0.00 0.00 Paved 0.00 | | |
| Travel Time (min) | = 7.33 | + | 5.80 | + | 0.00 | = | 13.13 |
| Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft) | = 0.00 = 0.00 = 0.00 = 0.015 = 0.00 = 0.0 | | 0.00 0.00 0.00 0.015 0.00 0.0 | | 0.00 0.00 0.00 0.015 0.00 0.0 | | |
| Travel Time (min) | = 0.00 | + | 0.00 | + | 0.00 | = | 0.00 |
| Total Travel Time, Tc | | | | | | | 21.80 min |

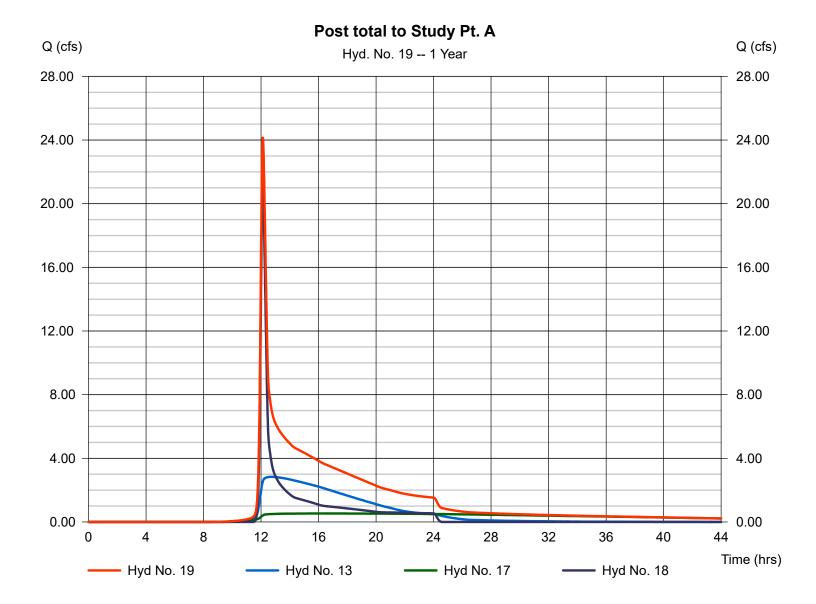
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Hyd. No. 19

Post total to Study Pt. A

Hydrograph type = Combine Storm frequency = 1 yrs Time interval = 1 min Inflow hyds. = 13, 17, 18 Peak discharge = 24.15 cfs Time to peak = 12.12 hrs Hyd. volume = 208,491 cuft Contrib. drain. area = 24.390 ac



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

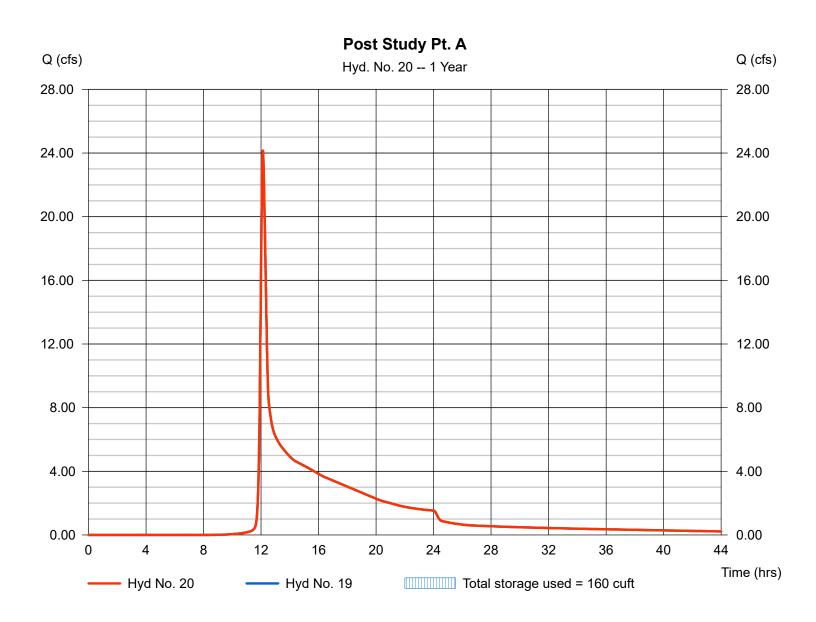
Monday, Jul 10, 2017

Hyd. No. 20

Post Study Pt. A

Hydrograph type = Reservoir Peak discharge = 24.16 cfsStorm frequency Time to peak = 1 yrs $= 12.13 \, hrs$ Time interval = 1 min Hyd. volume = 208,487 cuft Inflow hyd. No. = 19 - Post total to Study Pt. A Max. Elevation $= 970.45 \, \text{ft}$ Reservoir name = Ex. DS Culvert at Barclay Max. Storage = 160 cuft

Storage Indication method used.



Pond Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

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Pond No. 4 - Ex. DS Culvert at Barclay

Pond Data

Contours - User-defined contour areas. Conic method used for volume calculation. Begining Elevation = 968.76 ft

Stage / Storage Table

| Stage (ft) | Elevation (ft) | Contour area (sqft) | Incr. Storage (cuft) | Total storage (cuft) |
|------------|----------------|---------------------|----------------------|----------------------|
| 0.00 | 968.76 | 00 | 0 | 0 |
| 1.24 | 970.00 | 108 | 45 | 45 |
| 3.24 | 972.00 | 450 | 519 | 564 |
| 5.24 | 974.00 | 980 | 1,396 | 1,959 |
| 7.24 | 976.00 | 3,970 | 4,615 | 6,574 |
| 9.24 | 978.00 | 9,530 | 13,099 | 19,673 |
| 11.24 | 980.00 | 13,858 | 23,251 | 42,924 |
| 13.24 | 982.00 | 18,108 | 31,868 | 74,793 |

Culvert / Orifice Structures Weir Structures [A] [B] [C] [PrfRsr] [A] [B] [C] [D] Rise (in) = 54.00 0.00 0.00 0.00 Crest Len (ft) = 0.00 0.00 0.00 0.00 Span (in) = 54.00 0.00 0.00 0.00 Crest El. (ft) = 0.00 0.00 0.00 0.00 No. Barrels = 1 0 0 Weir Coeff. = 3.330 3.33 3.33 3.33 Invert El. (ft) = 968.76 0.00 0.00 0.00 Weir Type = ---Length (ft) = 0.000.00 0.00 0.00 Multi-Stage = No No No No Slope (%) = 0.000.00 0.00 n/a = .013 .013 .013 n/a N-Value Orifice Coeff. = 0.600.60 0.60 0.60 Exfil.(in/hr) = 0.000 (by Wet area) Multi-Stage = n/a No TW Elev. (ft) = 0.00

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Stage / Storage / Discharge Table

| Stage ft | Storage cuft | Elevation ft | Clv A cfs | Clv B cfs | CIv C cfs | PrfRsr cfs | Wr A cfs | Wr B cfs | Wr C cfs | Wr D cfs | Exfil cfs | User cfs | Total cfs |
|-------------|-----------------|-----------------|--------------|--------------|--------------|---------------|-------------|-------------|-------------|-------------|--------------|-------------|--------------|
| 0.00 | 0 | 968.76 | 0.00 | | | | | | | | | | 0.000 |
| 1.24 | 45 | 970.00 | 13.53 ic | | | | | | | | | | 13.53 |
| 3.24 | 564 | 972.00 | 75.20 ic | | | | | | | | | | 75.20 |
| 5.24 | 1,959 | 974.00 | 132.40 ic | | | | | | | | | | 132.40 |
| 7.24 | 6,574 | 976.00 | 171.04 ic | | | | | | | | | | 171.04 |
| 9.24 | 19,673 | 978.00 | 202.44 ic | | | | | | | | | | 202.44 |
| 11.24 | 42,924 | 980.00 | 229.58 ic | | | | | | | | | | 229.58 |
| 13.24 | 74,793 | 982.00 | 253.84 ic | | | | | | | | | | 253.84 |

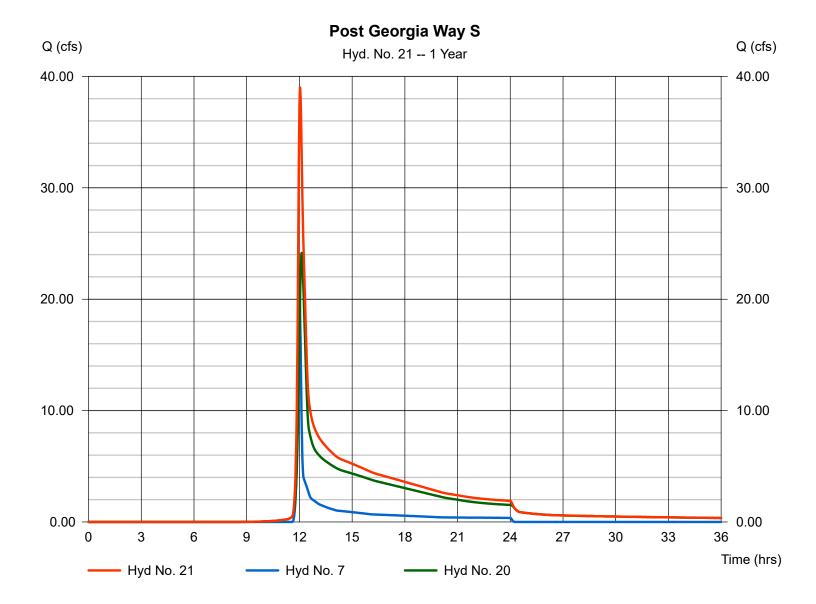
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Monday, Jul 10, 2017

Hyd. No. 21

Post Georgia Way S

Hydrograph type = Combine Storm frequency = 1 yrs Time interval = 1 min Inflow hyds. = 7, 20 Peak discharge = 39.01 cfs Time to peak = 12.03 hrs Hyd. volume = 258,408 cuft Contrib. drain. area = 17.800 ac



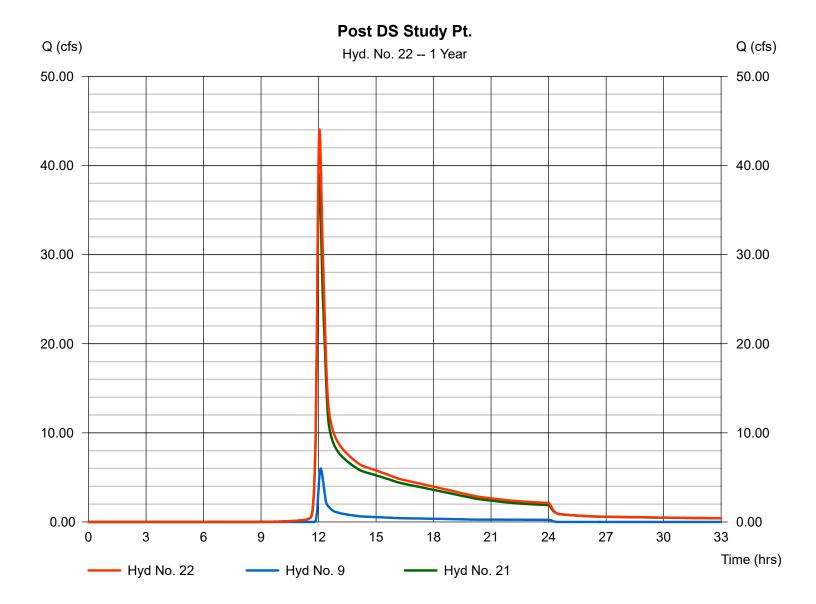
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 22

Post DS Study Pt.

Hydrograph type = Combine Storm frequency = 1 yrs Time interval = 1 min Inflow hyds. = 9, 21 Peak discharge = 44.04 cfs Time to peak = 12.05 hrs Hyd. volume = 284,689 cuft Contrib. drain. area = 15.500 ac



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

| | , , , , , , , , , , , , , , , , , , , | | | | | | drographs Exter | ision for AutoCAI | D® Civil 3D® 2009 by Autodesk, Inc. v6 |
|-------------|---------------------------------------|-----------------------|---------------------------|--------------------|--------------------------|------------------|------------------------------|-------------------------------|--|
| Hyd. No. | Hydrograph type (origin) | Peak flow (cfs) | Time interval (min) | Time to peak (min) | Hyd. volume (cuft) | Inflow hyd(s) | Maximum elevation (ft) | Total strge used (cuft) | Hydrograph description |
| 1 | SCS Runoff | 44.13 | 1 | 721 | 111,536 | | | | Pre Basin A2- to Ex. Detention Pond |
| 2 | Reservoir | 3.429 | 1 | 775 | 111,512 | 1 | 986.50 | 79,666 | Pre Ex. Pond |
| 3 | SCS Runoff | 6.173 | 1 | 723 | 18,778 | | | | Pre Basin A1- site |
| 4 | SCS Runoff | 32.72 | 1 | 727 | 113,592 | | | | Pre Basin A3 - bypass |
| 5 | Combine | 40.91 | 1 | 726 | 243,882 | 2, 3, 4 | | | Pre total to Study Pt. A |
| 6 | Reservoir | 40.90 | 1 | 726 | 243,882 | 5 | 971.02 | 308 | Pre Study Pt. A |
| 7 | SCS Runoff | 33.15 | 1 | 720 | 77,168 | | | | Offsite Basin 1 |
| 8 | Combine | 67.18 | 1 | 722 | 321,049 | 6, 7 | | | Pre Georgia Way S |
| 9 | SCS Runoff | 12.36 | 1 | 726 | 44,580 | | | | Offsite Basin 2 |
| 10 | Combine | 78.00 | 1 | 722 | 365,630 | 8, 9 | | | Pre DS Study Pt. |
| 12 | SCS Runoff | 42.49 | 1 | 721 | 107,391 | | | | Post Basin A2 to Ex. Pond |
| 13 | Reservoir | 3.397 | 1 | 768 | 107,367 | 12 | 986.42 | 78,560 | Post Ex. Pond |
| 14 | SCS Runoff | 25.73 | 1 | 723 | 73,722 | | | | Post Basin A1.1 - to prop. pond |
| 15 | SCS Runoff | 0.660 | 1 | 718 | 1,438 | | | | Post Basin A1.2 - to prop. pond |
| 16 | Combine | 26.10 | 1 | 723 | 75,159 | 14, 15 | | | Post total to prop. pond |
| 17 | Reservoir | 1.551 | 1 | 808 | 69,834 | 16 | 982.20 | 45,051 | Prop. pond |
| 18 | SCS Runoff | 33.96 | 1 | 727 | 117,894 | | | | Post Basin A3 - bypass |
| 19 | Combine | 37.50 | 1 | 727 | 295,095 | 13, 17, 18 | | | Post total to Study Pt. A |
| 20 | Reservoir | 37.48 | 1 | 727 | 295,091 | 19 | 970.91 | 280 | Post Study Pt. A |
| 21 | Combine | 62.49 | 1 | 722 | 372,258 | 7, 20 | | | Post Georgia Way S |
| 22 | Combine | 73.31 | 1 | 722 | 416,839 | 9, 21 | | | Post DS Study Pt. |
| 24 | Reservoir | 1.552 | 1 | 808 | 36,403 | 16 | 982.45 | 47,248 | Emergency Overflow |
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| | <u> </u> | 1 | 1 | 1 | | | | | 1 |

07-11-17.gpw Return Period: 2 Year Monday, Jul 10, 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 1

Storm duration

Pre Basin A2- to Ex. Detention Pond

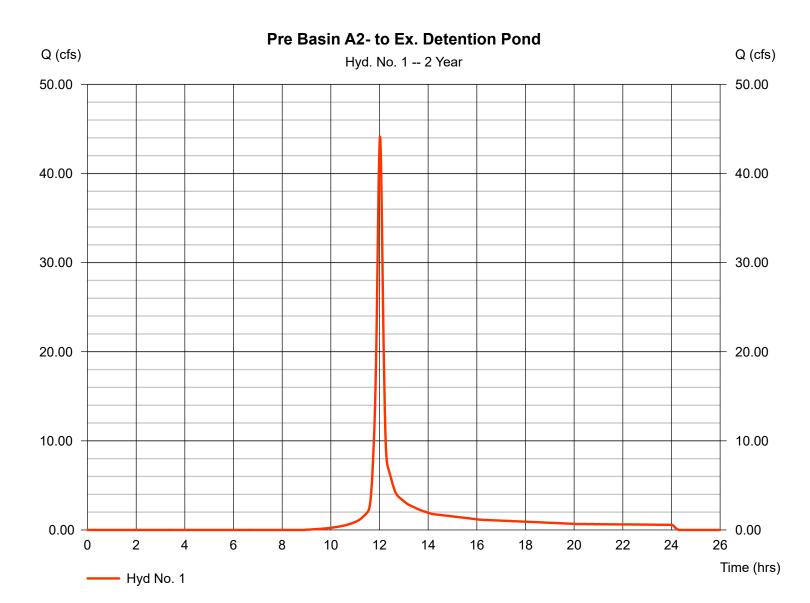
Hydrograph type = SCS Runoff
Storm frequency = 2 yrs
Time interval = 1 min
Drainage area = 16.000 ac
Basin Slope = 0.0 %
Tc method = TR55
Total precip. = 4.08 in

= 24 hrs

Peak discharge = 44.13 cfs
Time to peak = 12.02 hrs
Hyd. volume = 111,536 cuft
Curve number = 78

Curve number = 78Hydraulic length = 0 ft

Time of conc. (Tc) = 12.40 min
Distribution = Type II
Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

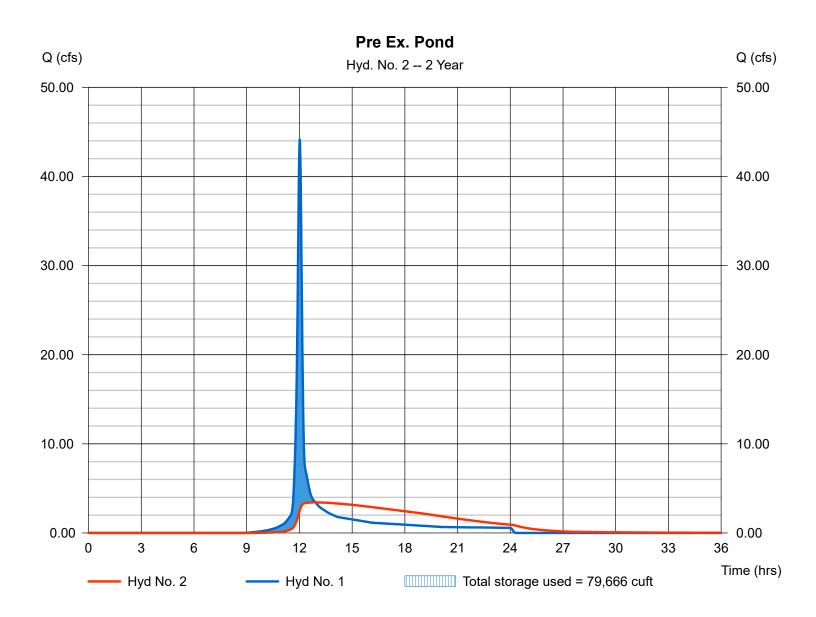
Monday, Jul 10, 2017

Hyd. No. 2

Pre Ex. Pond

Hydrograph type = Reservoir Peak discharge = 3.429 cfsStorm frequency Time to peak = 2 yrs $= 12.92 \, hrs$ Time interval = 1 min Hyd. volume = 111,512 cuft Inflow hyd. No. = 1 - Pre Basin A2- to Ex. Detention Pond Max. Elevation $= 986.50 \, \text{ft}$ Reservoir name = Ex. Pond Max. Storage = 79,666 cuft

Storage Indication method used. Wet pond routing start elevation = 982.00 ft.



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

= 24 hrs

Monday, Jul 10, 2017

= 484

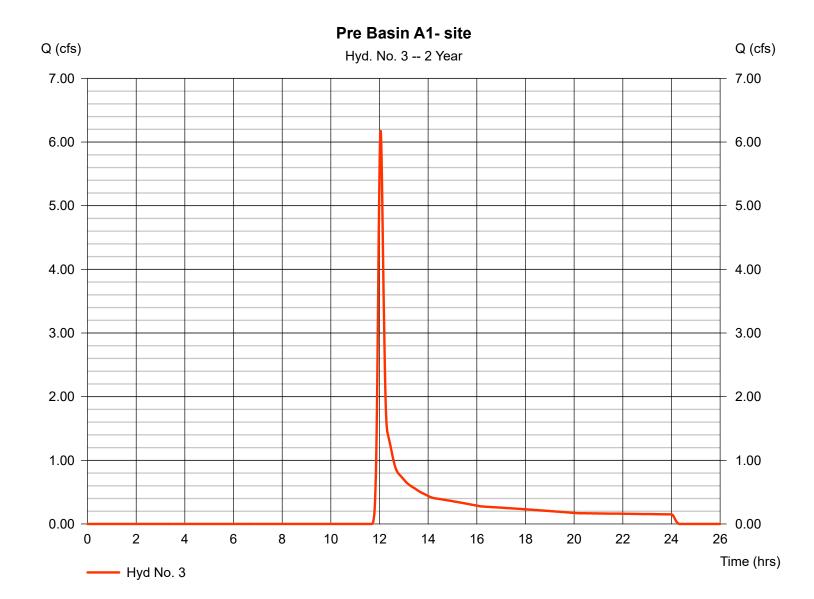
Hyd. No. 3

Pre Basin A1- site

Storm duration

Hydrograph type = SCS Runoff Peak discharge = 6.173 cfsStorm frequency Time to peak = 2 yrs $= 12.05 \, hrs$ Time interval = 1 min Hyd. volume = 18,778 cuft Drainage area = 7.000 acCurve number = 59 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 12.80 \, \text{min}$ Distribution Total precip. = 4.08 in= Type II

Shape factor



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 4

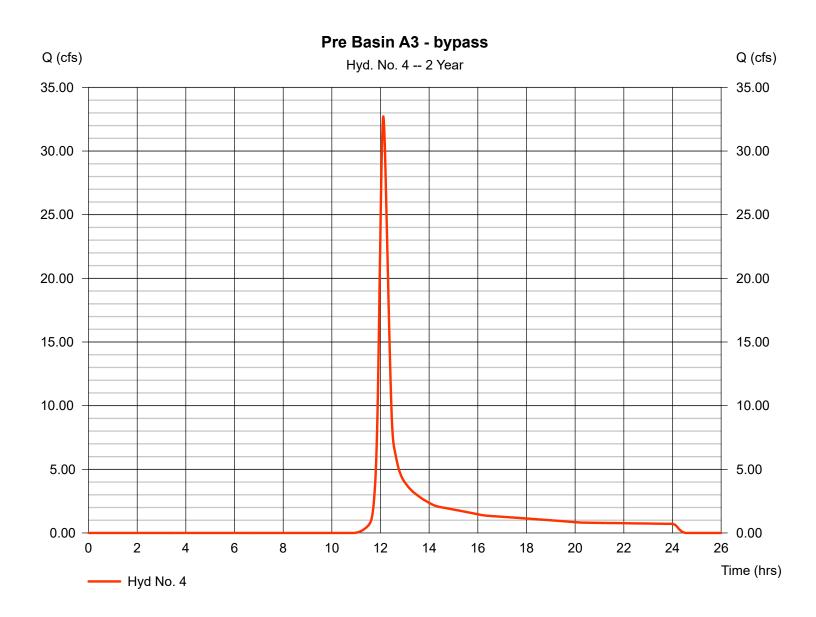
Pre Basin A3 - bypass

Hydrograph type = SCS Runoff Storm frequency = 2 yrsTime interval = 1 min Drainage area = 23.500 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 4.08 inStorm duration = 24 hrs

Peak discharge = 32.72 cfs
Time to peak = 12.12 hrs
Hyd. volume = 113,592 cuft
Curve number = 69

Curve number = 69 Hydraulic length = 0 ft

Time of conc. (Tc) = 21.80 min
Distribution = Type II
Shape factor = 484



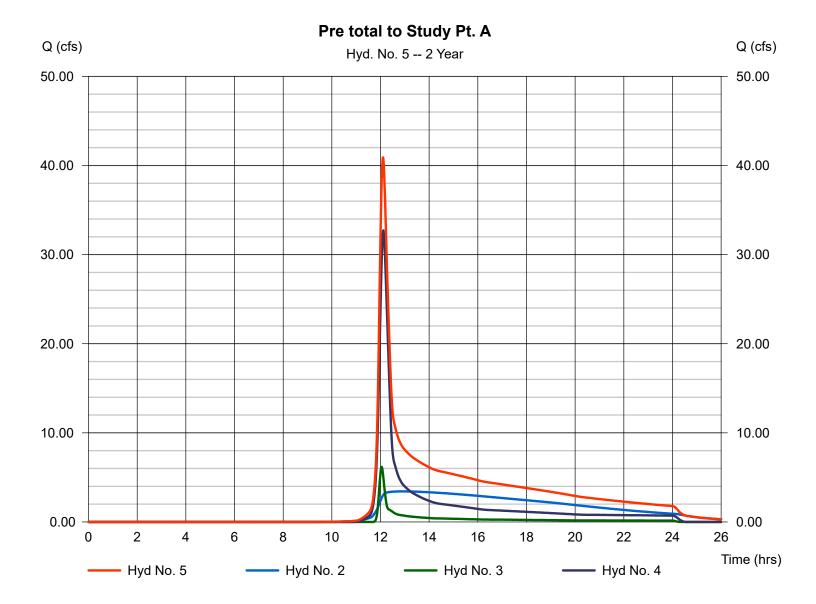
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 5

Pre total to Study Pt. A

Hydrograph type = Combine Storm frequency = 2 yrs Time interval = 1 min Inflow hyds. = 2, 3, 4 Peak discharge = 40.91 cfs Time to peak = 12.10 hrs Hyd. volume = 243,882 cuft Contrib. drain. area = 30.500 ac



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

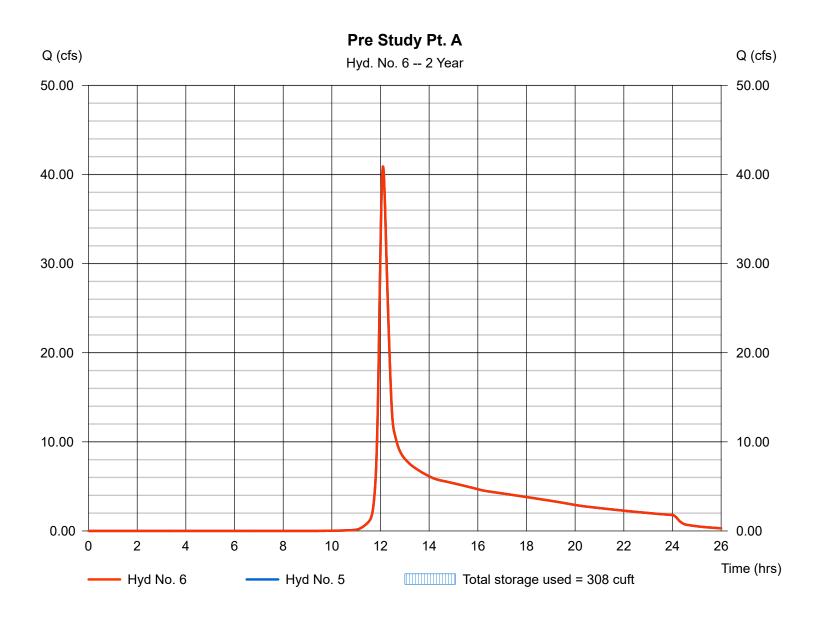
Monday, Jul 10, 2017

Hyd. No. 6

Pre Study Pt. A

Hydrograph type = Reservoir Peak discharge = 40.90 cfsStorm frequency Time to peak = 2 yrs = 12.10 hrsTime interval = 1 min Hyd. volume = 243,882 cuft Inflow hyd. No. = 5 - Pre total to Study Pt. A Max. Elevation = 971.02 ftReservoir name = Ex. DS Culvert at Barclay Max. Storage = 308 cuft

Storage Indication method used.



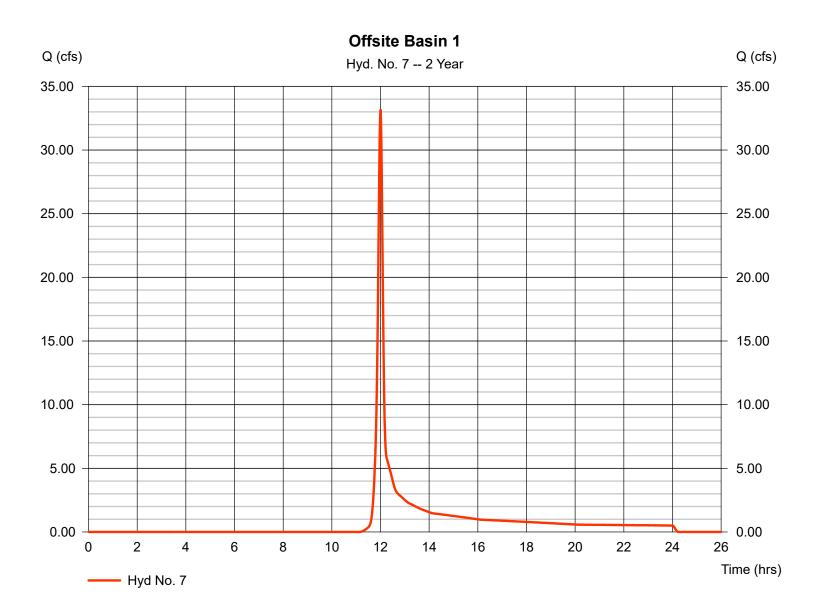
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 7

Offsite Basin 1

Hydrograph type = SCS Runoff Peak discharge = 33.15 cfsStorm frequency Time to peak = 2 yrs $= 12.00 \, hrs$ Time interval = 1 min Hyd. volume = 77,168 cuft Drainage area = 17.800 acCurve number = 67 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.80 \, \text{min}$ Distribution Total precip. = 4.08 in= Type II Storm duration = 24 hrs Shape factor = 484



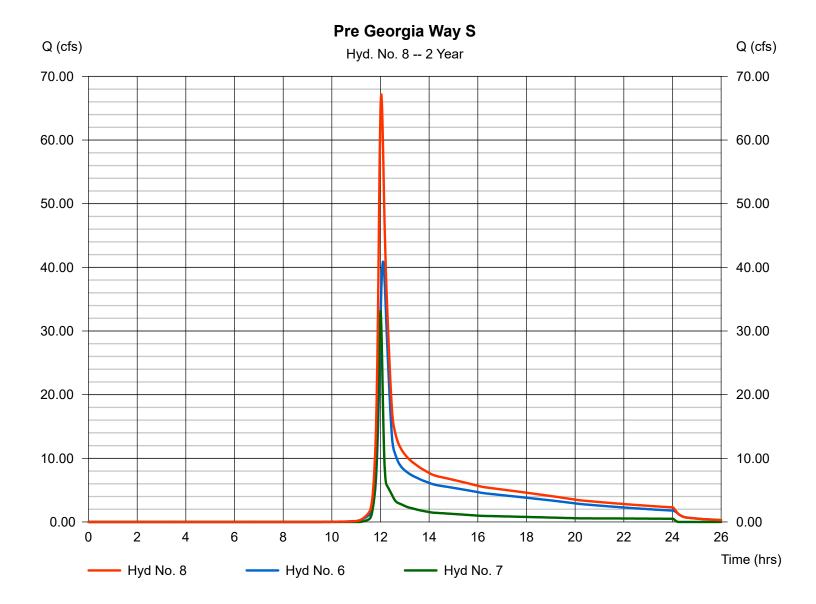
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 8

Pre Georgia Way S

Hydrograph type = Combine Storm frequency = 2 yrs Time interval = 1 min Inflow hyds. = 6, 7 Peak discharge = 67.18 cfs Time to peak = 12.03 hrs Hyd. volume = 321,049 cuft Contrib. drain. area = 17.800 ac



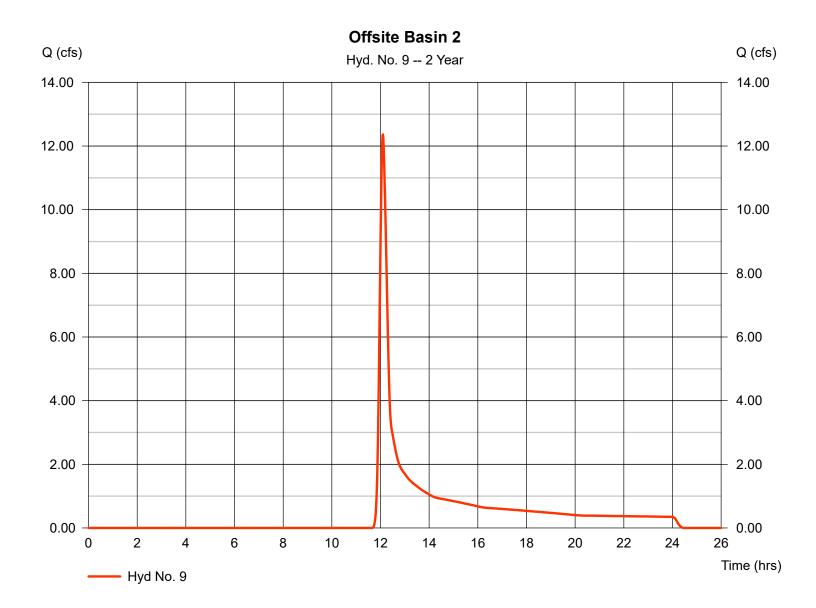
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 9

Offsite Basin 2

Hydrograph type = SCS Runoff Peak discharge = 12.36 cfsStorm frequency Time to peak = 2 yrs= 12.10 hrsTime interval = 1 min Hyd. volume = 44,580 cuftDrainage area = 15.500 acCurve number = 60 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 17.30 \, \text{min}$ Distribution Total precip. = 4.08 in= Type II Storm duration = 24 hrs Shape factor = 484



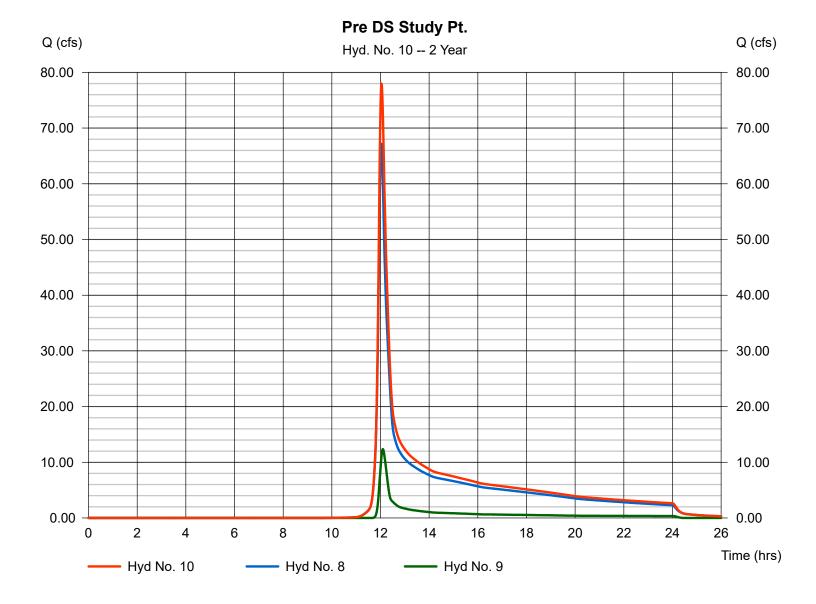
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Monday, Jul 10, 2017

Hyd. No. 10

Pre DS Study Pt.

Hydrograph type = Combine Storm frequency = 2 yrs Time interval = 1 min Inflow hyds. = 8, 9 Peak discharge = 78.00 cfs Time to peak = 12.03 hrs Hyd. volume = 365,630 cuft Contrib. drain. area = 15.500 ac



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Monday, Jul 10, 2017

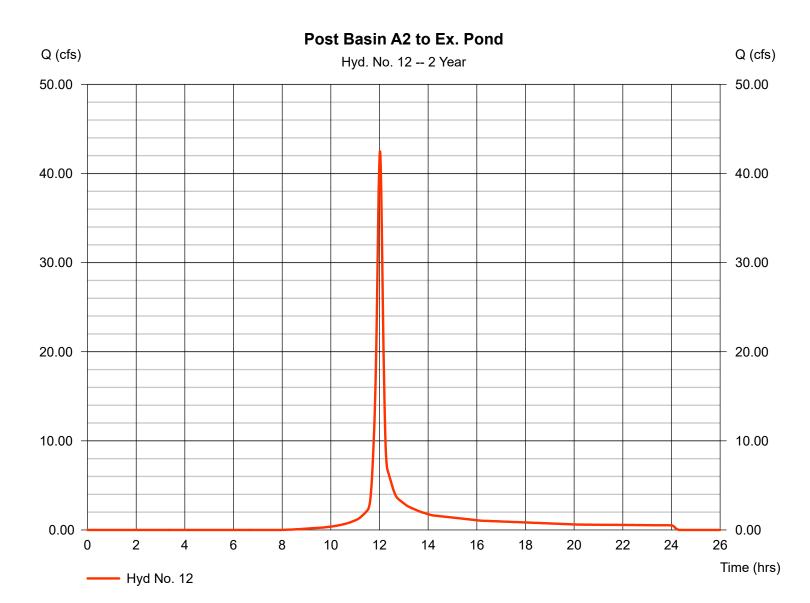
Hyd. No. 12

Post Basin A2 to Ex. Pond

Hydrograph type = SCS Runoff Storm frequency = 2 yrsTime interval = 1 min Drainage area = 13.730 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 4.08 inStorm duration = 24 hrs

Peak discharge = 42.49 cfs
Time to peak = 12.02 hrs
Hyd. volume = 107,391 cuft
Curve number = 81
Hydraulic length = 0 ft

Time of conc. (Tc) = 12.40 min
Distribution = Type II
Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

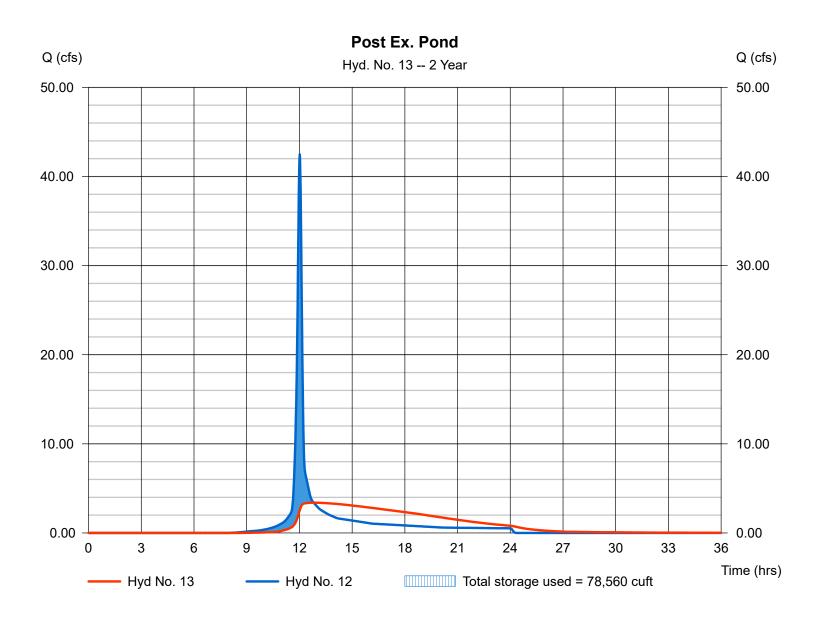
Monday, Jul 10, 2017

Hyd. No. 13

Post Ex. Pond

Hydrograph type = Reservoir Peak discharge = 3.397 cfsStorm frequency Time to peak = 2 yrs $= 12.80 \, hrs$ Time interval = 1 min Hyd. volume = 107,367 cuft Inflow hyd. No. = 12 - Post Basin A2 to Ex. Pond Max. Elevation = 986.42 ftReservoir name = Ex. Pond Max. Storage = 78,560 cuft

Storage Indication method used. Wet pond routing start elevation = 982.00 ft.



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Monday, Jul 10, 2017

Hyd. No. 14

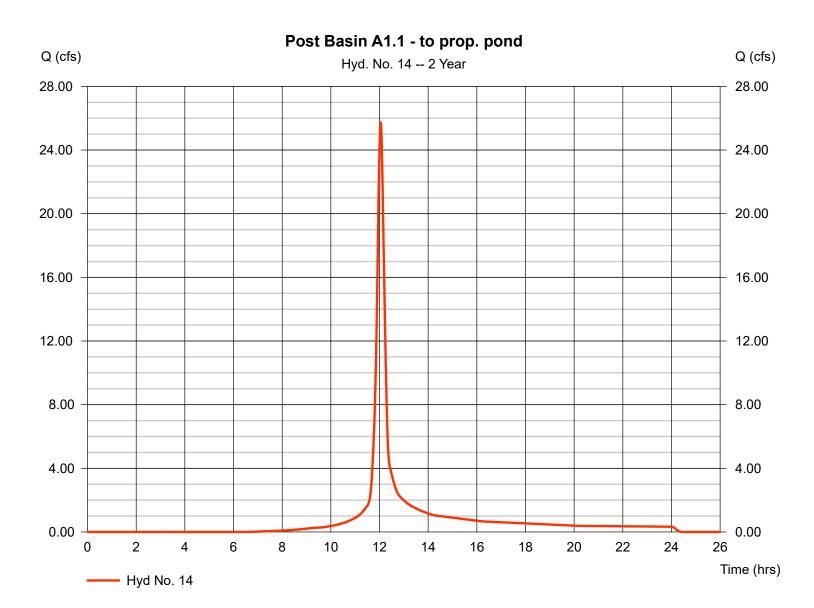
Post Basin A1.1 - to prop. pond

Hydrograph type = SCS Runoff Storm frequency = 2 yrsTime interval = 1 min Drainage area = 7.930 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 4.08 inStorm duration = 24 hrs

Peak discharge = 25.73 cfs
Time to peak = 12.05 hrs
Hyd. volume = 73,722 cuft
Curve number = 85

Curve number = 85 Hydraulic length = 0 ft Time of conc. (Tc) = 16.30 min

Distribution = Type II Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Q (cfs)

= 0.660 cfs

 $= 11.97 \, hrs$

= 1,438 cuft

 $= 5.00 \, \text{min}$

= Type II

= 61

= 0 ft

= 484

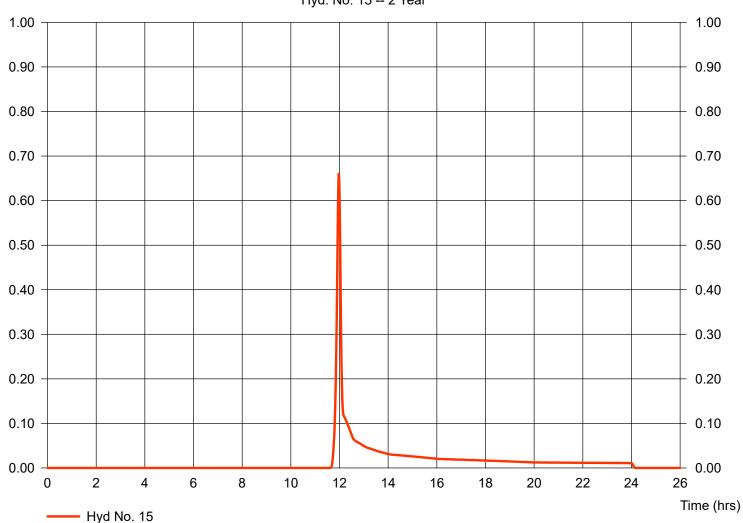
Hyd. No. 15

Q (cfs)

Post Basin A1.2 - to prop. pond

Hydrograph type = SCS Runoff Peak discharge Storm frequency Time to peak = 2 yrs Time interval = 1 min Hyd. volume Drainage area = 0.450 acCurve number Basin Slope = 0.0 % Hydraulic length Tc method = USER Time of conc. (Tc) Distribution Total precip. = 4.08 inStorm duration = 24 hrs Shape factor

Post Basin A1.2 - to prop. pond Hyd. No. 15 -- 2 Year



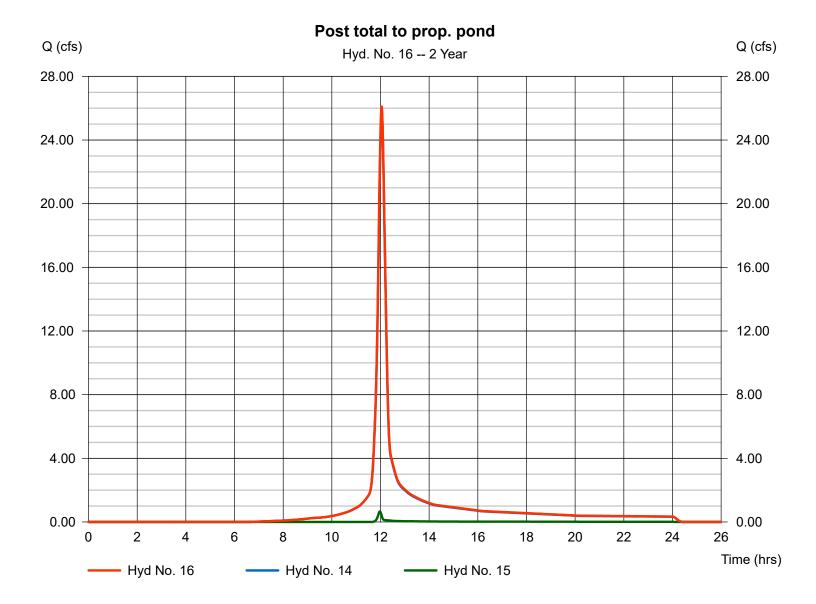
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 16

Post total to prop. pond

Hydrograph type = Combine Storm frequency = 2 yrs Time interval = 1 min Inflow hyds. = 14, 15 Peak discharge = 26.10 cfs Time to peak = 12.05 hrs Hyd. volume = 75,159 cuft Contrib. drain. area = 8.380 ac



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

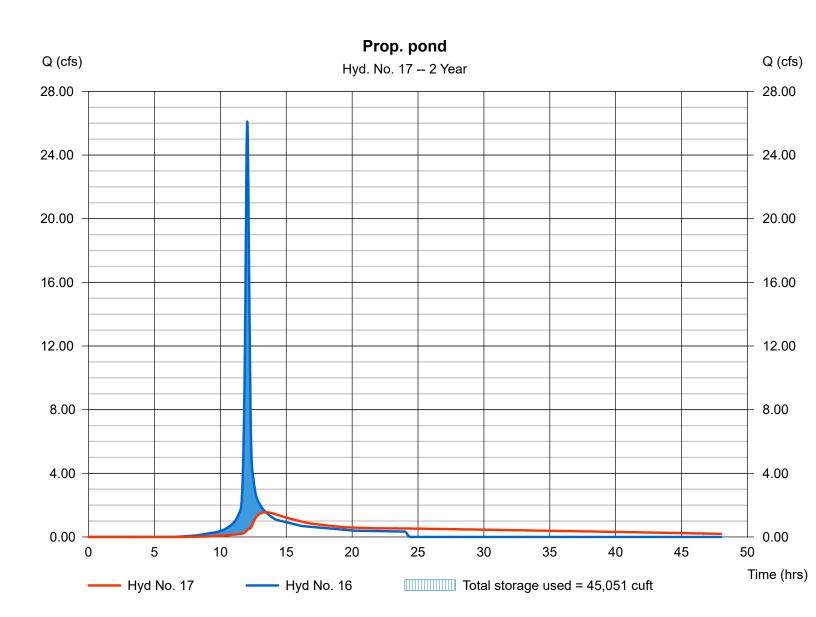
Monday, Jul 10, 2017

Hyd. No. 17

Prop. pond

Hydrograph type = Reservoir Peak discharge = 1.551 cfsStorm frequency Time to peak = 2 yrs $= 13.47 \, hrs$ Time interval = 1 min Hyd. volume = 69,834 cuft Inflow hyd. No. = 16 - Post total to prop. pond Max. Elevation = 982.20 ftReservoir name = Stormwater Pond Max. Storage = 45,051 cuft

Storage Indication method used.



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 18

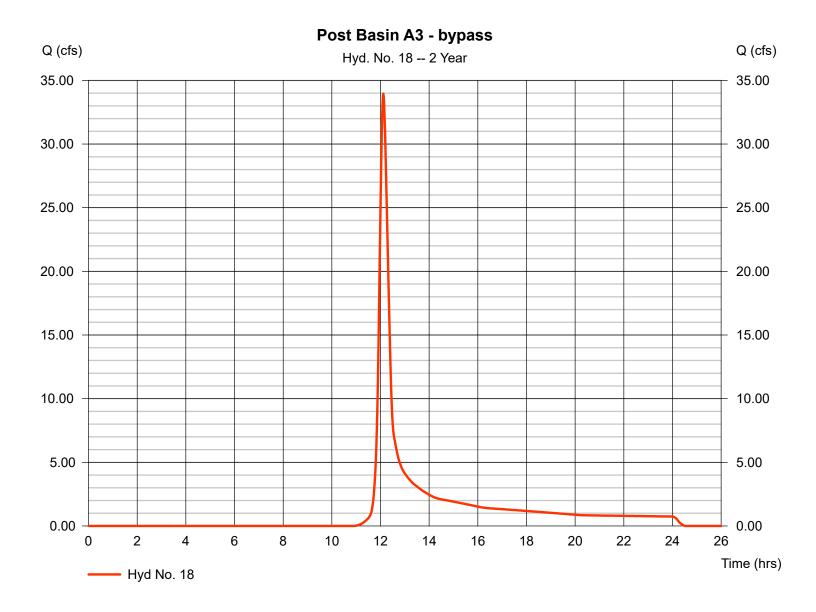
Post Basin A3 - bypass

Hydrograph type = SCS Runoff Storm frequency = 2 yrsTime interval = 1 min Drainage area = 24.390 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 4.08 inStorm duration = 24 hrs

Peak discharge = 33.96 cfs
Time to peak = 12.12 hrs
Hyd. volume = 117,894 cuft
Curve number = 69

Hydraulic length = 0 ft
Time of conc. (Tc) = 21.80 min

Distribution = Type II Shape factor = 484



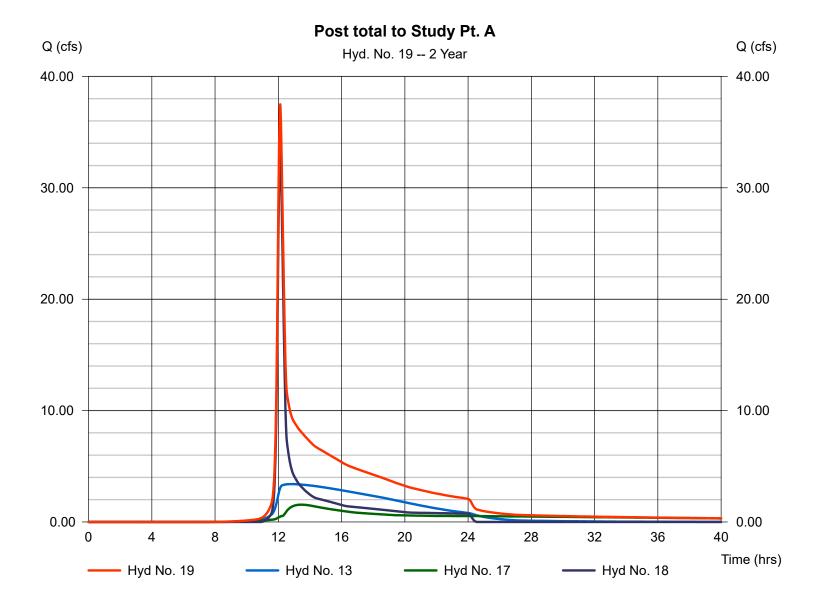
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 19

Post total to Study Pt. A

Hydrograph type = Combine Storm frequency = 2 yrs Time interval = 1 min Inflow hyds. = 13, 17, 18 Peak discharge = 37.50 cfs Time to peak = 12.12 hrs Hyd. volume = 295,095 cuft Contrib. drain. area = 24.390 ac



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

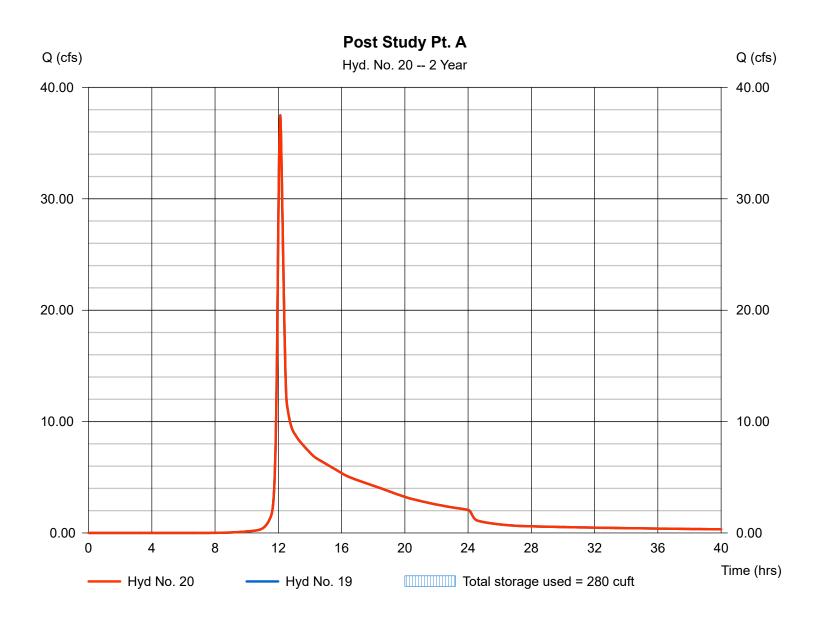
Monday, Jul 10, 2017

Hyd. No. 20

Post Study Pt. A

Hydrograph type = Reservoir Peak discharge = 37.48 cfsStorm frequency Time to peak = 2 yrs $= 12.12 \, hrs$ Time interval = 1 min Hyd. volume = 295,091 cuftInflow hyd. No. = 19 - Post total to Study Pt. A Max. Elevation $= 970.91 \, \text{ft}$ Reservoir name = Ex. DS Culvert at Barclay Max. Storage = 280 cuft

Storage Indication method used.



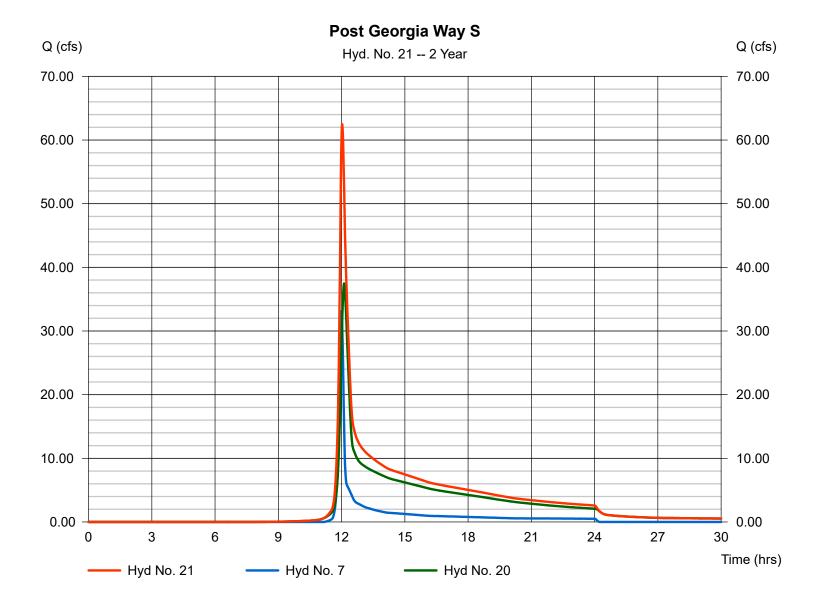
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Monday, Jul 10, 2017

Hyd. No. 21

Post Georgia Way S

Hydrograph type = Combine Storm frequency = 2 yrs Time interval = 1 min Inflow hyds. = 7, 20 Peak discharge = 62.49 cfs Time to peak = 12.03 hrs Hyd. volume = 372,258 cuft Contrib. drain. area = 17.800 ac



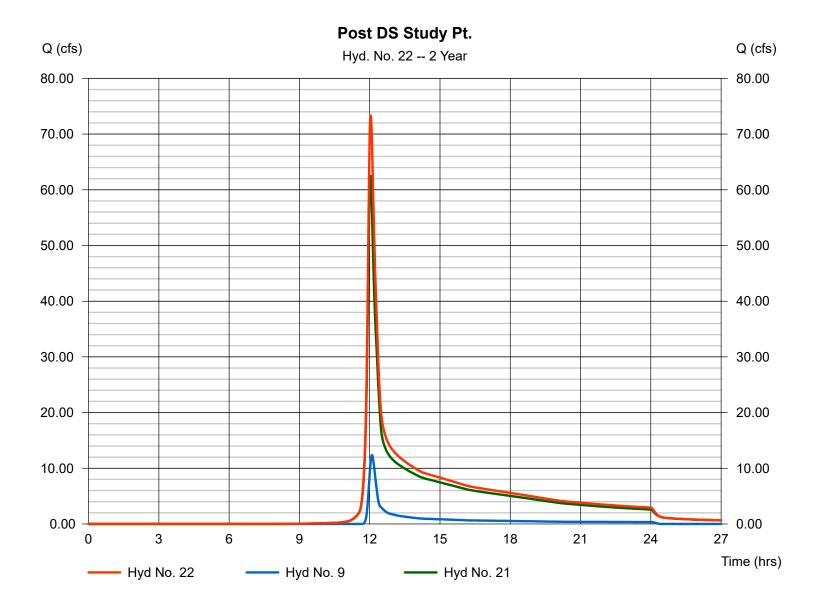
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 22

Post DS Study Pt.

Hydrograph type = Combine Storm frequency = 2 yrs Time interval = 1 min Inflow hyds. = 9, 21 Peak discharge = 73.31 cfs Time to peak = 12.03 hrs Hyd. volume = 416,839 cuft Contrib. drain. area = 15.500 ac



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

| Hyd. No. | Hydrograph type (origin) | Peak flow (cfs) | Time interval (min) | Time to peak (min) | Hyd. volume (cuft) | Inflow hyd(s) | Maximum elevation (ft) | Total strge used (cuft) | Hydrograph description |
|-------------|--------------------------------|-----------------------|---------------------------|--------------------------|--------------------------|------------------|------------------------------|-------------------------------|-------------------------------------|
| 1 | SCS Runoff | 57.53 | 1 | 721 | 145,377 | | | | Pre Basin A2- to Ex. Detention Pond |
| 2 | Reservoir | 3.956 | 1 | 782 | 145,345 | 1 | 987.87 | 99,427 | Pre Ex. Pond |
| 3 | SCS Runoff | 10.09 | 1 | 722 | 28,079 | | | | Pre Basin A1- site |
| 4 | SCS Runoff | 46.17 | 1 | 727 | 156,172 | | | | Pre Basin A3 - bypass |
| 5 | Combine | 58.01 | 1 | 726 | 329,596 | 2, 3, 4 | | | Pre total to Study Pt. A |
| 6 | Reservoir | 58.02 | 1 | 726 | 329,596 | 5 | 971.53 | 440 | Pre Study Pt. A |
| 7 | SCS Runoff | 47.09 | 1 | 720 | 107,590 | | | | Offsite Basin 1 |
| 8 | Combine | 96.31 | 1 | 722 | 437,186 | 6, 7 | | | Pre Georgia Way S |
| 9 | SCS Runoff | 19.97 | 1 | 725 | 65,970 | | | | Offsite Basin 2 |
| 10 | Combine | 114.53 | 1 | 722 | 503,156 | 8, 9 | | | Pre DS Study Pt. |
| 12 | SCS Runoff | 54.32 | 1 | 721 | 137,827 | | | | Post Basin A2 to Ex. Pond |
| 13 | Reservoir | 3.874 | 1 | 776 | 137,798 | 12 | 987.65 | 96,159 | Post Ex. Pond |
| 14 | SCS Runoff | 32.18 | 1 | 723 | 92,791 | | | | Post Basin A1.1 - to prop. pond |
| 15 | SCS Runoff | 1.014 | 1 | 718 | 2,107 | | | | Post Basin A1.2 - to prop. pond |
| 16 | Combine | 32.73 | 1 | 723 | 94,898 | 14, 15 | | | Post total to prop. pond |
| 17 | Reservoir | 3.692 | 1 | 757 | 89,031 | 16 | 983.00 | 51,999 | Prop. pond |
| 18 | SCS Runoff | 47.92 | 1 | 727 | 162,087 | | | | Post Basin A3 - bypass |
| 19 | Combine | 51.97 | 1 | 727 | 388,916 | 13, 17, 18 | | | Post total to Study Pt. A |
| 20 | Reservoir | 51.97 | 1 | 727 | 388,911 | 19 | 971.35 | 395 | Post Study Pt. A |
| 21 | Combine | 88.28 | 1 | 721 | 496,501 | 7, 20 | | | Post Georgia Way S |
| 22 | Combine | 106.42 | 1 | 722 | 562,472 | 9, 21 | | | Post DS Study Pt. |
| 24 | Reservoir | 3.825 | 1 | 756 | 56,137 | 16 | 983.24 | 54,029 | Emergency Overflow |
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07-11-17.gpw Return Period: 5 Year Monday, Jul 10, 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 1

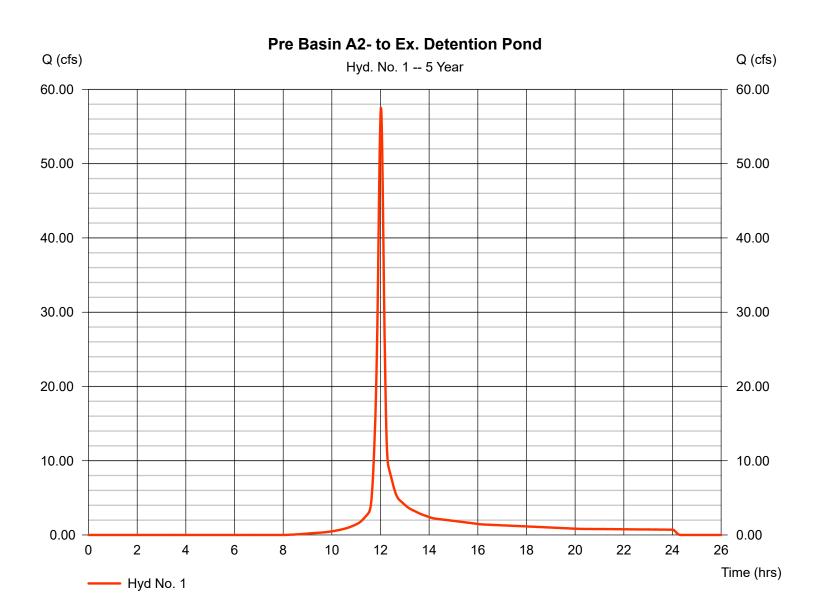
Pre Basin A2- to Ex. Detention Pond

= SCS Runoff Hydrograph type Storm frequency = 5 yrsTime interval = 1 min Drainage area = 16.000 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 4.80 inStorm duration = 24 hrs

Peak discharge = 57.53 cfs
Time to peak = 12.02 hrs
Hyd. volume = 145,377 cuft
Curve number = 78

Curve number = 78 Hydraulic length = 0 ft

Time of conc. (Tc) = 12.40 min
Distribution = Type II
Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

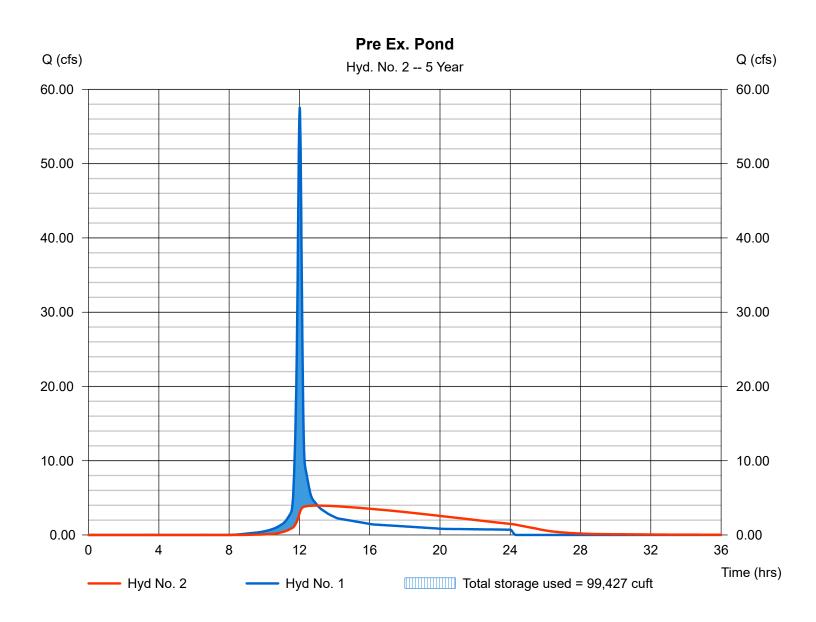
Monday, Jul 10, 2017

Hyd. No. 2

Pre Ex. Pond

Hydrograph type = Reservoir Peak discharge = 3.956 cfsStorm frequency Time to peak = 5 yrs $= 13.03 \, hrs$ Time interval = 1 min Hyd. volume = 145,345 cuft Inflow hyd. No. = 1 - Pre Basin A2- to Ex. Detention Pond Max. Elevation = 987.87 ftReservoir name = Ex. Pond Max. Storage = 99,427 cuft

Storage Indication method used. Wet pond routing start elevation = 982.00 ft.



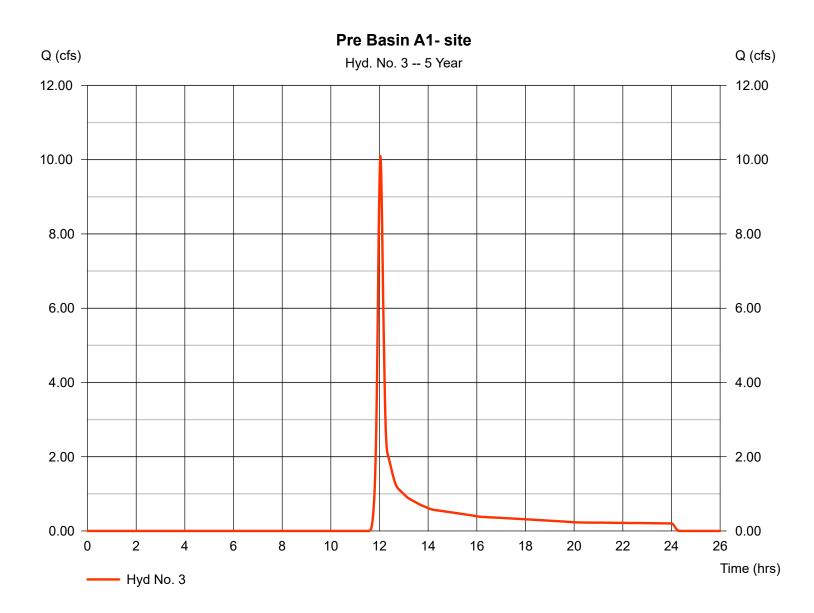
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 3

Pre Basin A1- site

Hydrograph type = SCS Runoff Peak discharge = 10.09 cfsStorm frequency Time to peak = 5 yrs $= 12.03 \, hrs$ Time interval = 1 min Hyd. volume = 28,079 cuftDrainage area = 7.000 acCurve number = 59 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 12.80 \, \text{min}$ Distribution Total precip. = 4.80 in= Type II Storm duration = 484 = 24 hrs Shape factor



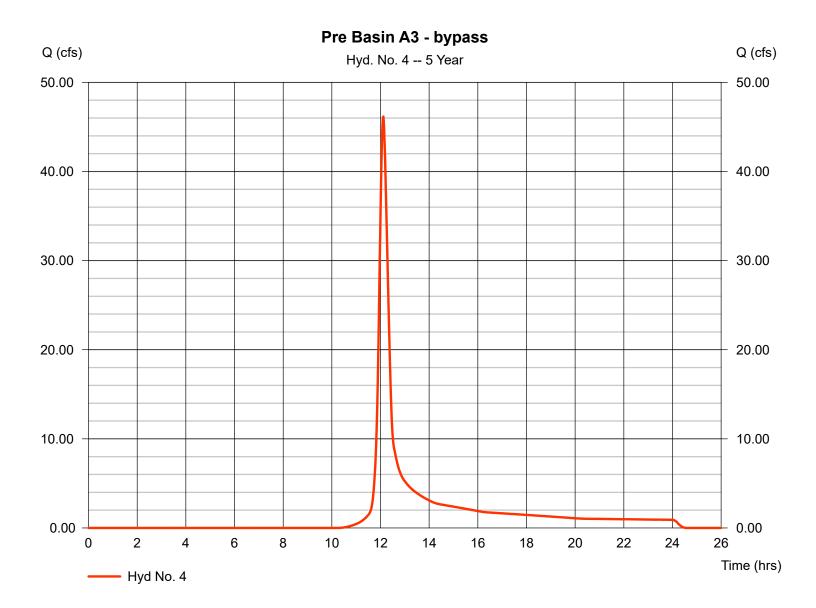
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 4

Pre Basin A3 - bypass

Hydrograph type = SCS Runoff Peak discharge = 46.17 cfsStorm frequency Time to peak = 5 yrs $= 12.12 \, hrs$ Time interval = 1 min Hyd. volume = 156,172 cuft Drainage area = 23.500 acCurve number = 69 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 21.80 \, \text{min}$ Total precip. = 4.80 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



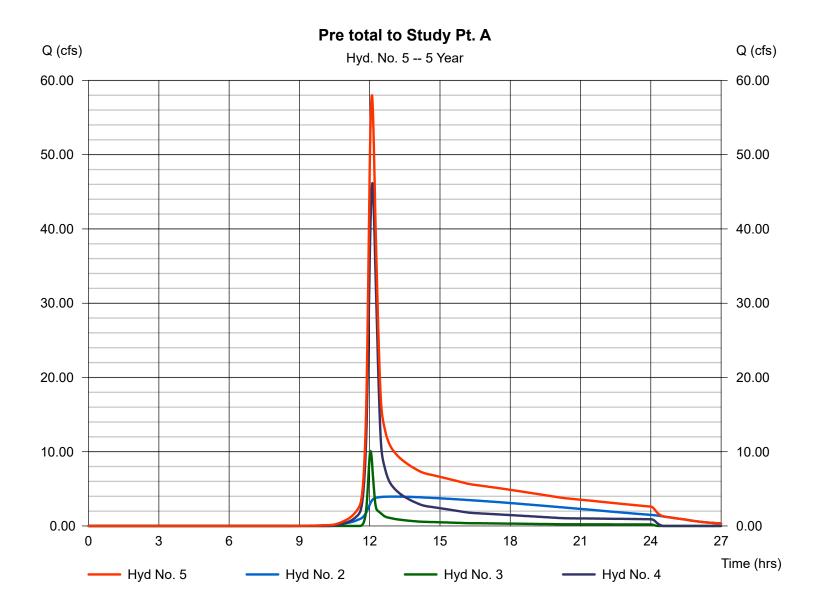
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 5

Pre total to Study Pt. A

Hydrograph type = Combine Storm frequency = 5 yrs Time interval = 1 min Inflow hyds. = 2, 3, 4 Peak discharge = 58.01 cfs Time to peak = 12.10 hrs Hyd. volume = 329,596 cuft Contrib. drain. area = 30.500 ac



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

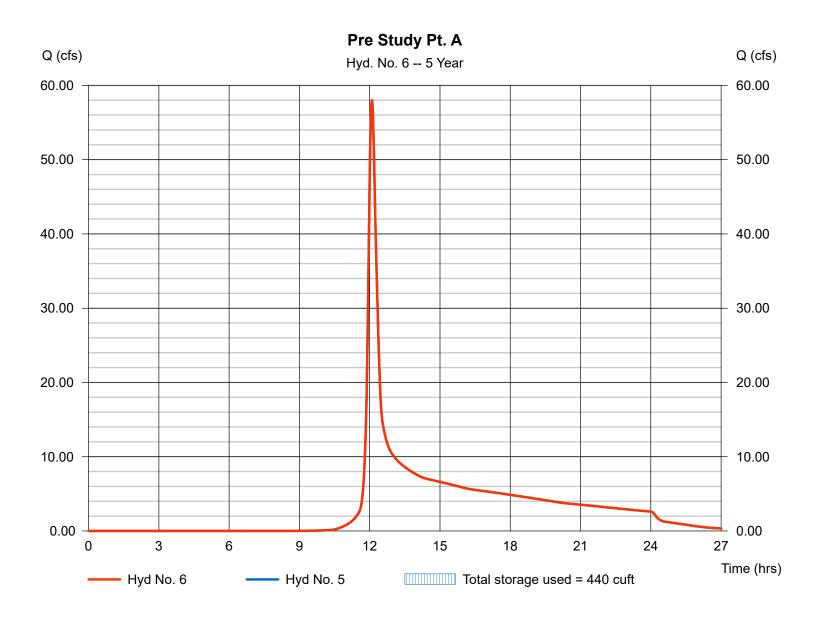
Monday, Jul 10, 2017

Hyd. No. 6

Pre Study Pt. A

Hydrograph type = Reservoir Peak discharge = 58.02 cfsStorm frequency Time to peak = 5 yrs= 12.10 hrsTime interval = 1 min Hyd. volume = 329,596 cuft Inflow hyd. No. = 5 - Pre total to Study Pt. A Max. Elevation = 971.53 ftReservoir name = Ex. DS Culvert at Barclay Max. Storage = 440 cuft

Storage Indication method used.



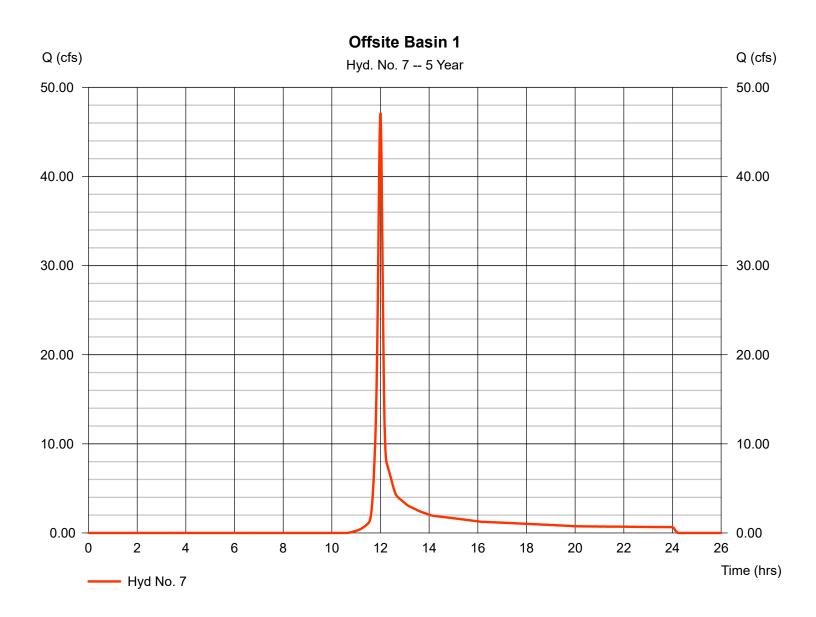
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 7

Offsite Basin 1

Hydrograph type = SCS Runoff Peak discharge = 47.09 cfsStorm frequency Time to peak = 5 yrs $= 12.00 \, hrs$ Time interval = 1 min Hyd. volume = 107,590 cuftDrainage area = 17.800 acCurve number = 67 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.80 \, \text{min}$ Total precip. = 4.80 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



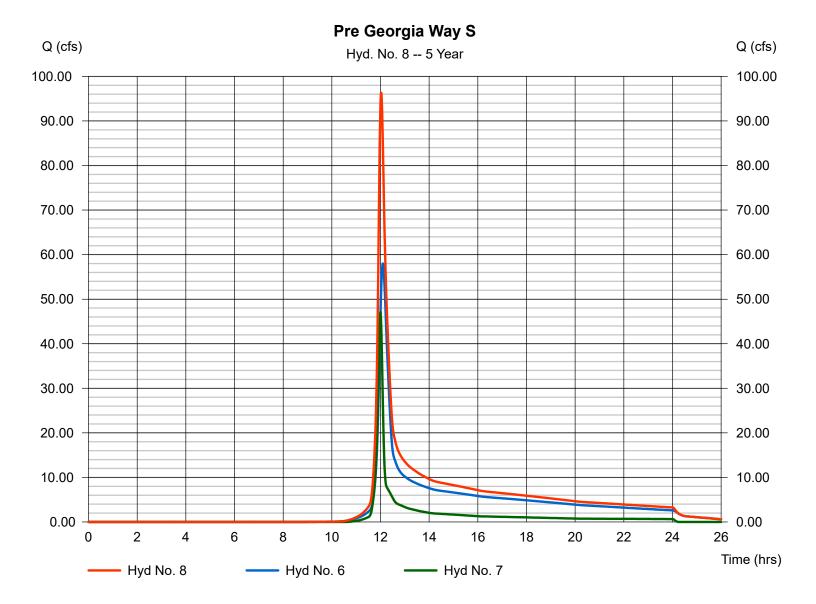
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Monday, Jul 10, 2017

Hyd. No. 8

Pre Georgia Way S

Hydrograph type = Combine Storm frequency = 5 yrs Time interval = 1 min Inflow hyds. = 6, 7 Peak discharge = 96.31 cfs Time to peak = 12.03 hrs Hyd. volume = 437,186 cuft Contrib. drain. area = 17.800 ac



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Monday, Jul 10, 2017

= 19.97 cfs

 $= 12.08 \, hrs$

= 17.30 min

= Type II

= 60

= 0 ft

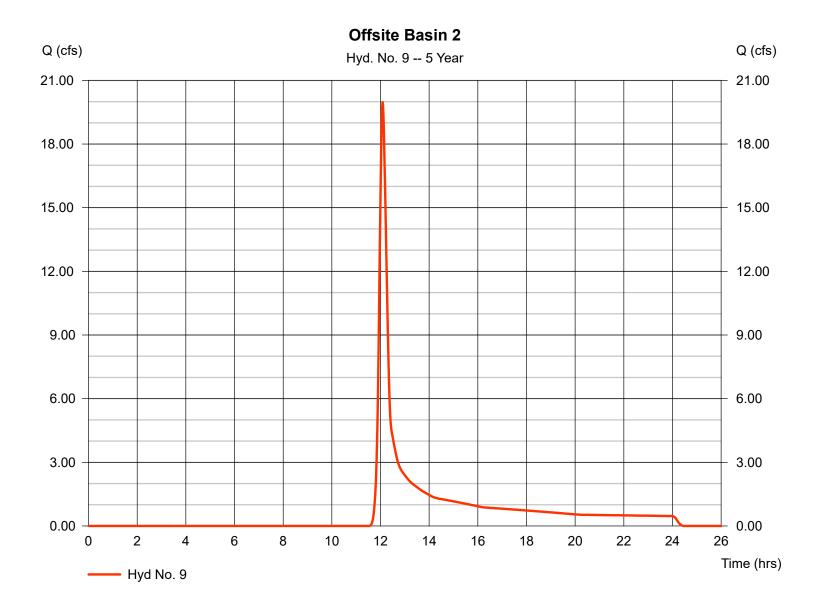
= 484

= 65,970 cuft

Hyd. No. 9

Offsite Basin 2

Hydrograph type = SCS Runoff Peak discharge Storm frequency Time to peak = 5 yrsTime interval = 1 min Hyd. volume Drainage area = 15.500 acCurve number Basin Slope = 0.0 % Hydraulic length Tc method = TR55 Time of conc. (Tc) Distribution Total precip. = 4.80 inStorm duration = 24 hrs Shape factor



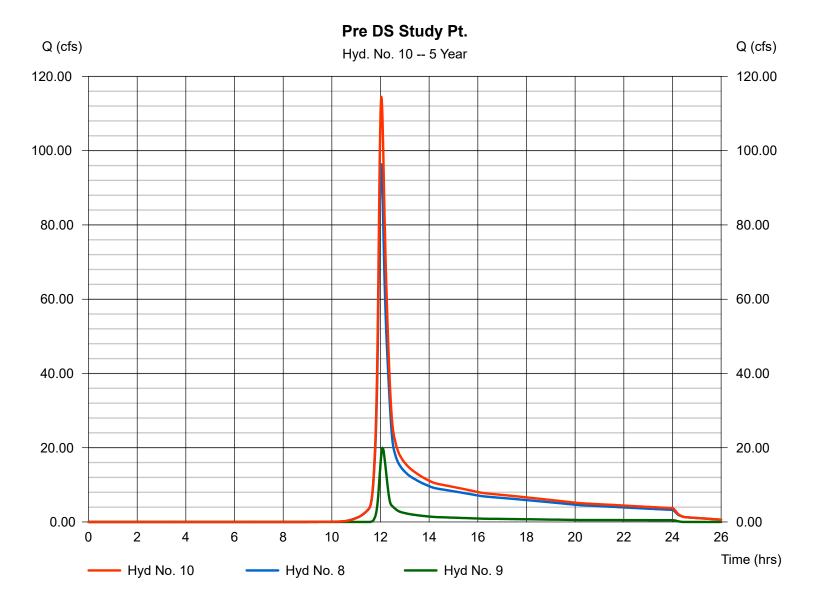
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Monday, Jul 10, 2017

Hyd. No. 10

Pre DS Study Pt.

Hydrograph type = Combine Storm frequency = 5 yrs Time interval = 1 min Inflow hyds. = 8, 9 Peak discharge = 114.53 cfs Time to peak = 12.03 hrs Hyd. volume = 503,156 cuft Contrib. drain. area = 15.500 ac



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Hyd. No. 12

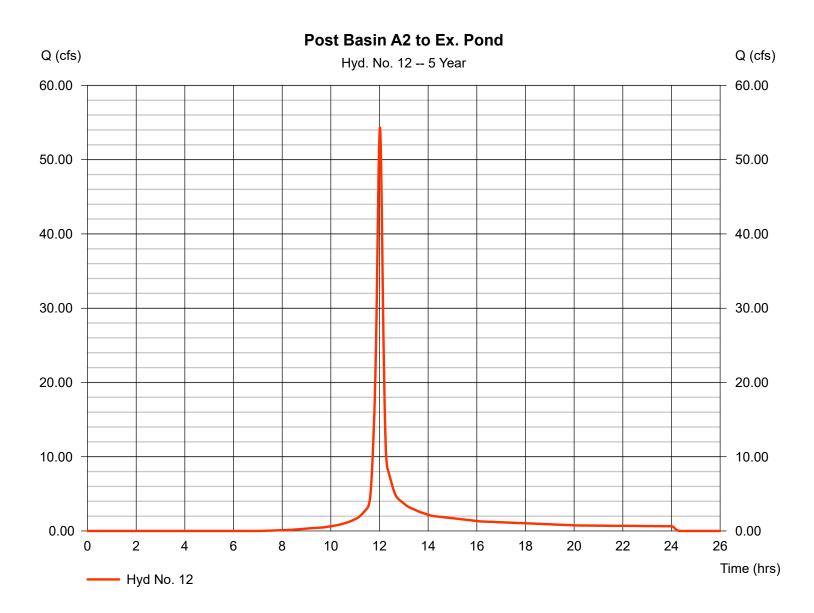
Post Basin A2 to Ex. Pond

Hydrograph type = SCS Runoff Storm frequency = 5 yrsTime interval = 1 min Drainage area = 13.730 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 4.80 inStorm duration = 24 hrs

Peak discharge = 54.32 cfs
Time to peak = 12.02 hrs
Hyd. volume = 137,827 cuft
Curve number = 81

Curve number = 81 Hydraulic length = 0 ft

Time of conc. (Tc) = 12.40 min
Distribution = Type II
Shape factor = 484



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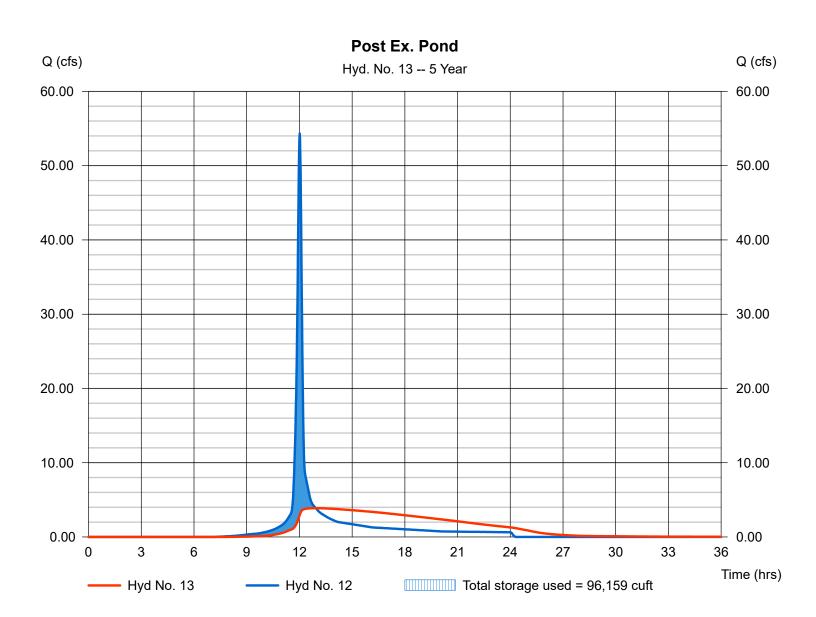
Monday, Jul 10, 2017

Hyd. No. 13

Post Ex. Pond

Hydrograph type = Reservoir Peak discharge = 3.874 cfsStorm frequency Time to peak = 5 yrs $= 12.93 \, hrs$ Time interval = 1 min Hyd. volume = 137,798 cuft Inflow hyd. No. = 12 - Post Basin A2 to Ex. Pond Max. Elevation $= 987.65 \, \mathrm{ft}$ Reservoir name = Ex. Pond Max. Storage = 96,159 cuft

Storage Indication method used. Wet pond routing start elevation = 982.00 ft.



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Monday, Jul 10, 2017

Hyd. No. 14

Storm duration

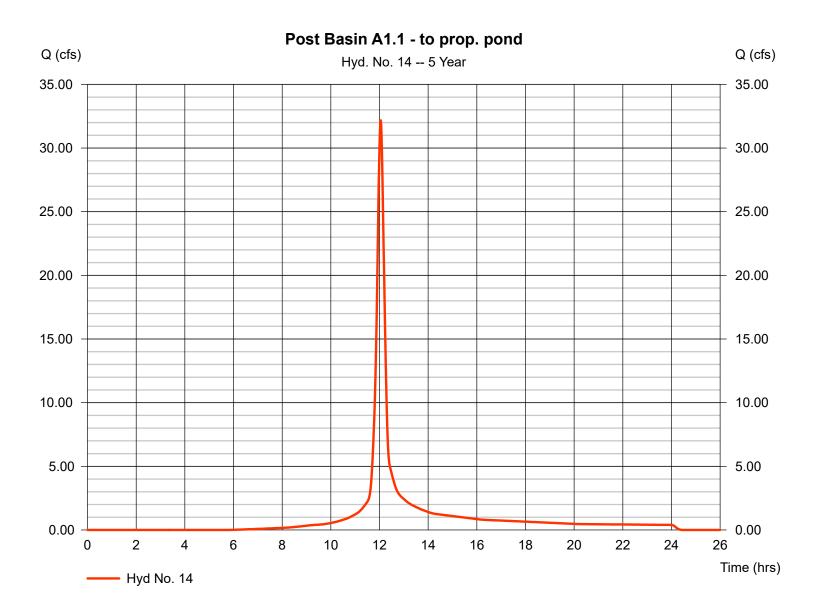
Post Basin A1.1 - to prop. pond

Hydrograph type = SCS Runoff
Storm frequency = 5 yrs
Time interval = 1 min
Drainage area = 7.930 ac
Basin Slope = 0.0 %
Tc method = TR55
Total precip. = 4.80 in

= 24 hrs

Peak discharge = 32.18 cfs
Time to peak = 12.05 hrs
Hyd. volume = 92,791 cuft
Curve number = 85

Hydraulic length = 0 ft
Time of conc. (Tc) = 16.30 min
Distribution = Type II
Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

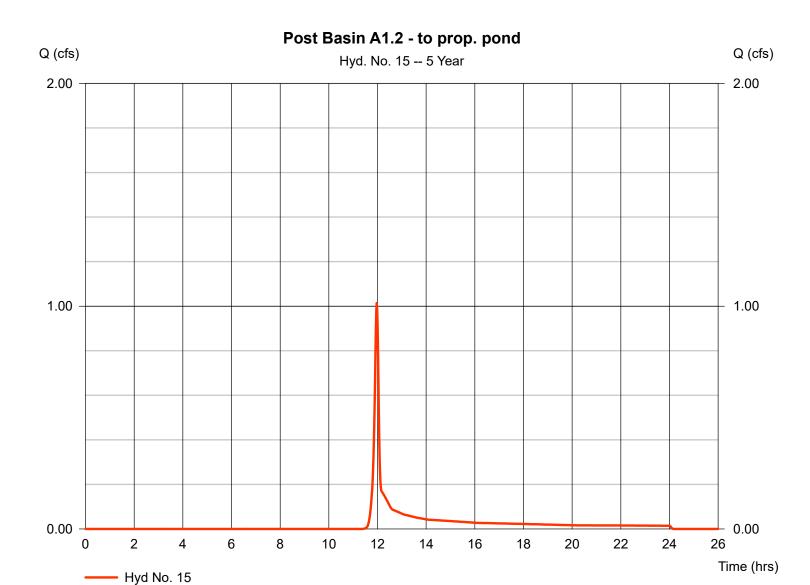
Monday, Jul 10, 2017

Hyd. No. 15

Post Basin A1.2 - to prop. pond

Hydrograph type = SCS Runoff Storm frequency = 5 yrsTime interval = 1 min Drainage area = 0.450 acBasin Slope = 0.0 % Tc method = USER Total precip. = 4.80 inStorm duration = 24 hrs

Peak discharge = 1.014 cfsTime to peak $= 11.97 \, hrs$ Hyd. volume = 2,107 cuftCurve number = 61 Hydraulic length = 0 ftTime of conc. (Tc) $= 5.00 \, \text{min}$ Distribution = Type II Shape factor = 484



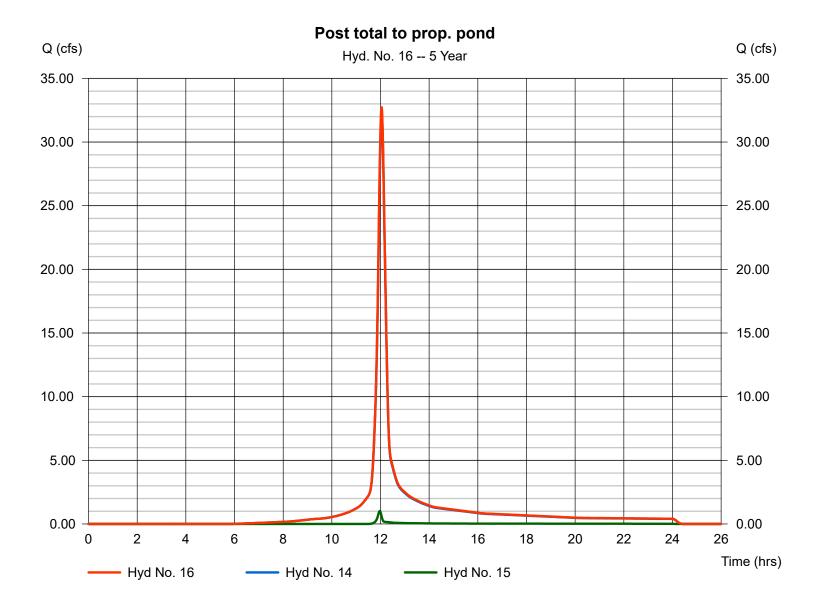
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Monday, Jul 10, 2017

Hyd. No. 16

Post total to prop. pond

Hydrograph type = Combine Storm frequency = 5 yrs Time interval = 1 min Inflow hyds. = 14, 15 Peak discharge = 32.73 cfs Time to peak = 12.05 hrs Hyd. volume = 94,898 cuft Contrib. drain. area = 8.380 ac



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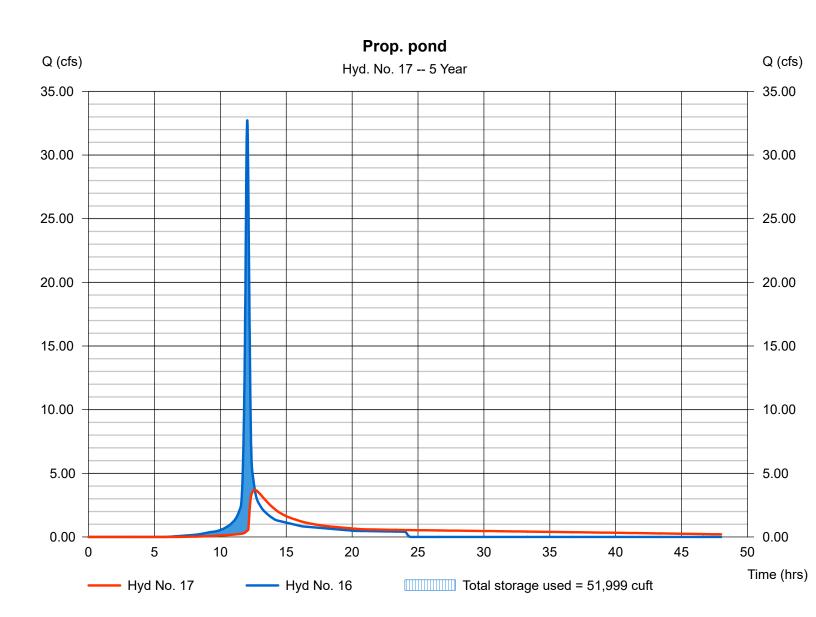
Monday, Jul 10, 2017

Hyd. No. 17

Prop. pond

Hydrograph type = Reservoir Peak discharge = 3.692 cfsStorm frequency Time to peak = 5 yrs $= 12.62 \, hrs$ Time interval = 1 min Hyd. volume = 89,031 cuft Max. Elevation Inflow hyd. No. = 16 - Post total to prop. pond $= 983.00 \, \text{ft}$ Reservoir name = Stormwater Pond Max. Storage = 51,999 cuft

Storage Indication method used.



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Hyd. No. 18

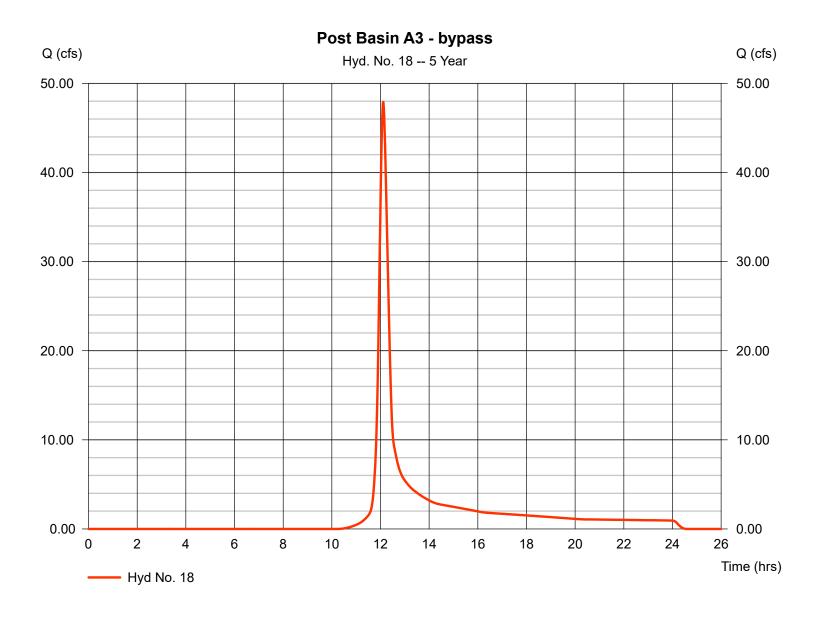
Post Basin A3 - bypass

Hydrograph type = SCS Runoff Storm frequency = 5 yrsTime interval = 1 min Drainage area = 24.390 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 4.80 inStorm duration = 24 hrs

Peak discharge = 47.92 cfs
Time to peak = 12.12 hrs
Hyd. volume = 162,087 cuft
Curve number = 69

Hydraulic length = 0 ft
Time of conc. (Tc) = 21.80 min
Distribution = Type II

Shape factor = 484



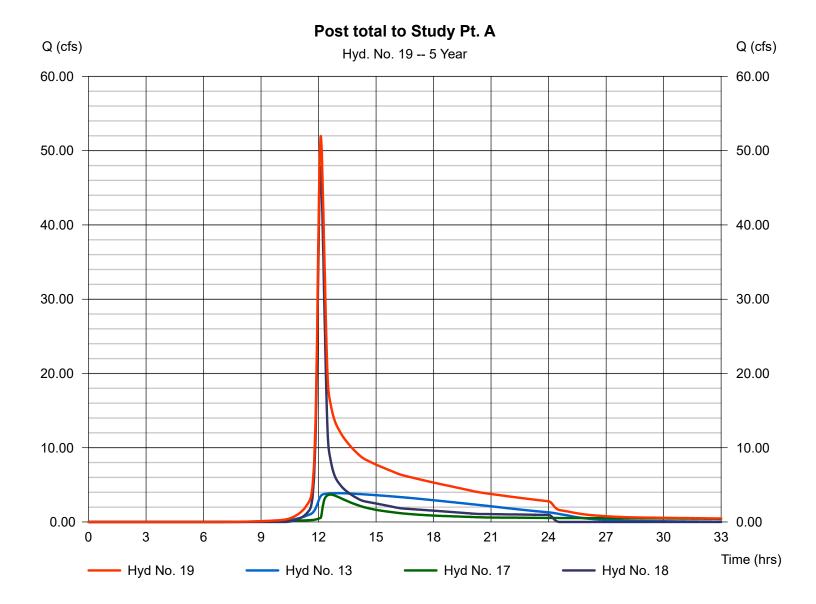
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Monday, Jul 10, 2017

Hyd. No. 19

Post total to Study Pt. A

Hydrograph type = Combine Storm frequency = 5 yrs Time interval = 1 min Inflow hyds. = 13, 17, 18 Peak discharge = 51.97 cfs
Time to peak = 12.12 hrs
Hyd. volume = 388,916 cuft
Contrib. drain. area = 24.390 ac



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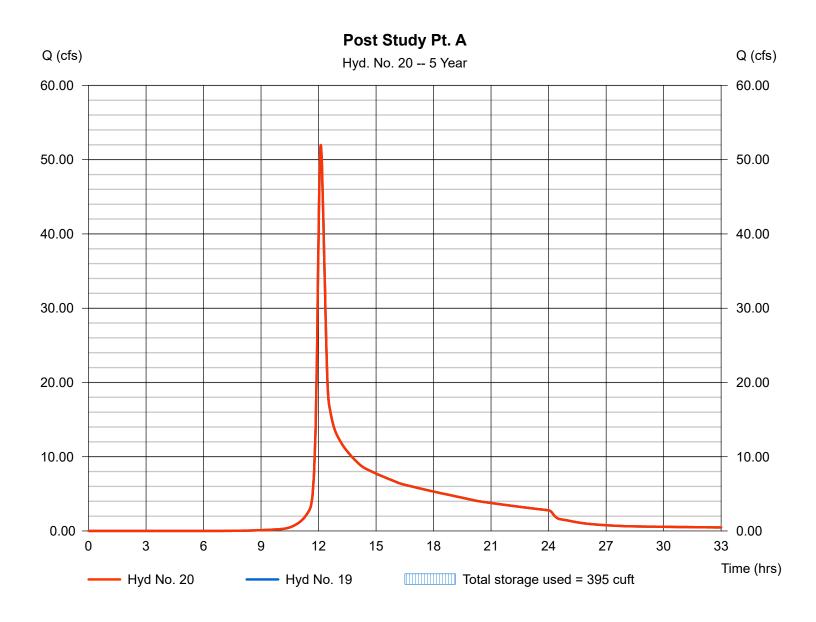
Monday, Jul 10, 2017

Hyd. No. 20

Post Study Pt. A

Hydrograph type = Reservoir Peak discharge = 51.97 cfsStorm frequency Time to peak = 5 yrs $= 12.12 \, hrs$ Time interval = 1 min Hyd. volume = 388,911 cuft Inflow hyd. No. = 19 - Post total to Study Pt. A Max. Elevation $= 971.35 \, ft$ Reservoir name = Ex. DS Culvert at Barclay Max. Storage = 395 cuft

Storage Indication method used.



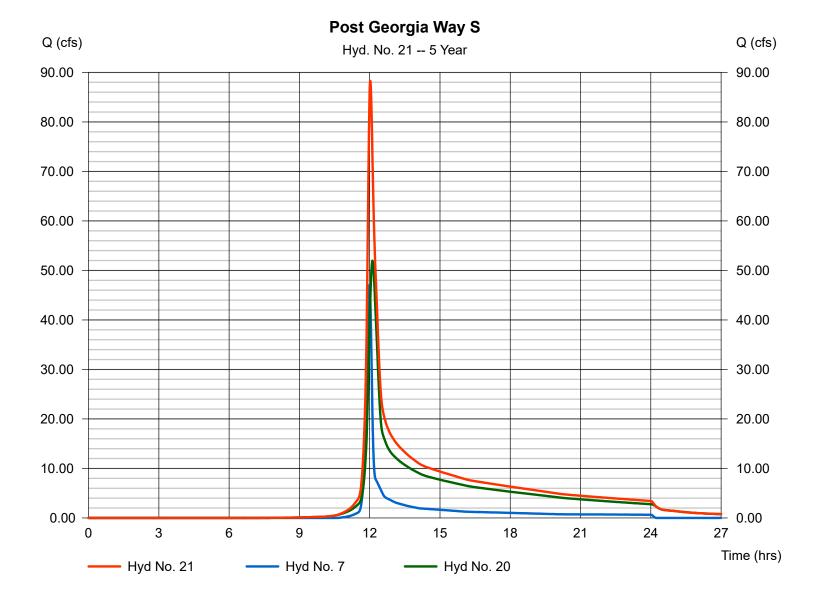
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Monday, Jul 10, 2017

Hyd. No. 21

Post Georgia Way S

Hydrograph type = Combine Storm frequency = 5 yrs Time interval = 1 min Inflow hyds. = 7, 20 Peak discharge = 88.28 cfs Time to peak = 12.02 hrs Hyd. volume = 496,501 cuft Contrib. drain. area = 17.800 ac



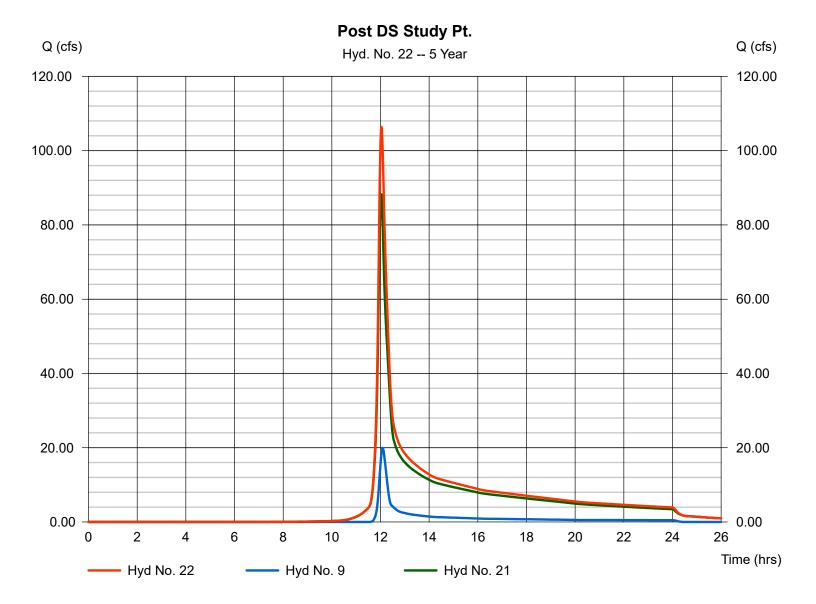
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Monday, Jul 10, 2017

Hyd. No. 22

Post DS Study Pt.

Hydrograph type = Combine Storm frequency = 5 yrs Time interval = 1 min Inflow hyds. = 9, 21 Peak discharge = 106.42 cfs Time to peak = 12.03 hrs Hyd. volume = 562,472 cuft Contrib. drain. area = 15.500 ac



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

| | | | | | • | Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v | | | | |
|-------------|--------------------------------|-----------------------|---------------------------|--------------------|--------------------------|---|------------------------------|-------------------------------|-------------------------------------|--|
| Hyd. No. | Hydrograph type (origin) | Peak flow (cfs) | Time interval (min) | Time to peak (min) | Hyd. volume (cuft) | Inflow hyd(s) | Maximum elevation (ft) | Total strge used (cuft) | Hydrograph description | |
| 1 | SCS Runoff | 71.26 | 1 | 721 | 180,573 | | | | Pre Basin A2- to Ex. Detention Pond | |
| 2 | Reservoir | 7.416 | 1 | 754 | 180,534 | 1 | 988.83 | 114,566 | Pre Ex. Pond | |
| 3 | SCS Runoff | 14.44 | 1 | 722 | 38,511 | | | | Pre Basin A1- site | |
| 4 | SCS Runoff | 60.46 | 1 | 726 | 201,822 | | | | Pre Basin A3 - bypass | |
| 5 | Combine | 76.36 | 1 | 725 | 420,868 | 2, 3, 4 | | | Pre total to Study Pt. A | |
| 6 | Reservoir | 76.32 | 1 | 726 | 420,867 | 5 | 972.04 | 585 | Pre Study Pt. A | |
| 7 | SCS Runoff | 61.94 | 1 | 720 | 140,463 | | | | Offsite Basin 1 | |
| 8 | Combine | 127.85 | 1 | 721 | 561,331 | 6, 7 | | | Pre Georgia Way S | |
| 9 | SCS Runoff | 28.43 | 1 | 725 | 89,836 | | | | Offsite Basin 2 | |
| 10 | Combine | 153.93 | 1 | 722 | 651,166 | 8, 9 | | | Pre DS Study Pt. | |
| 12 | SCS Runoff | 66.33 | 1 | 721 | 169,216 | | | | Post Basin A2 to Ex. Pond | |
| 13 | Reservoir | 6.232 | 1 | 757 | 169,180 | 12 | 988.62 | 111,178 | Post Ex. Pond | |
| 14 | SCS Runoff | 38.66 | 1 | 723 | 112,258 | | | | Post Basin A1.1 - to prop. pond | |
| 15 | SCS Runoff | 1.399 | 1 | 718 | 2,849 | | | | Post Basin A1.2 - to prop. pond | |
| 16 | Combine | 39.42 | 1 | 722 | 115,108 | 14, 15 | | | Post total to prop. pond | |
| 17 | Reservoir | 7.055 | 1 | 742 | 108,861 | 16 | 983.95 | 60,151 | Prop. pond | |
| 18 | SCS Runoff | 62.75 | 1 | 726 | 209,465 | | | | Post Basin A3 - bypass | |
| 19 | Combine | 69.21 | 1 | 727 | 487,506 | 13, 17, 18 | | | Post total to Study Pt. A | |
| 20 | Reservoir | 69.17 | 1 | 727 | 487,501 | 19 | 971.84 | 521 | Post Study Pt. A | |
| 21 | Combine | 116.00 | 1 | 721 | 627,965 | 7, 20 | | | Post Georgia Way S | |
| 22 | Combine | 142.00 | 1 | 722 | 717,801 | 9, 21 | | | Post DS Study Pt. | |
| 24 | Reservoir | 7.404 | 1 | 742 | 76,343 | 16 | 984.20 | 62,474 | Emergency Overflow | |
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07-11-17.gpw Return Period: 10 Year Monday, Jul 10, 2017

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Monday, Jul 10, 2017

Hyd. No. 1

Pre Basin A2- to Ex. Detention Pond

= SCS Runoff Hydrograph type Storm frequency = 10 yrsTime interval = 1 min Drainage area = 16.000 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 5.52 inStorm duration = 24 hrs

Peak discharge = 71.26 cfs
Time to peak = 12.02 hrs
Hyd. volume = 180,573 cuft
Curve number = 78

Curve number = 78 Hydraulic length = 0 ft Time of conc. (Tc) = 12.40 min

Distribution = Type II Shape factor = 484

Pre Basin A2- to Ex. Detention Pond Q (cfs) Q (cfs) Hyd. No. 1 -- 10 Year 80.00 00.08 70.00 70.00 60.00 60.00 50.00 50.00 40.00 40.00 30.00 30.00 20.00 20.00 10.00 10.00 0.00 0.00 2 6 8 10 12 14 16 18 20 22 24 26 Time (hrs) Hyd No. 1

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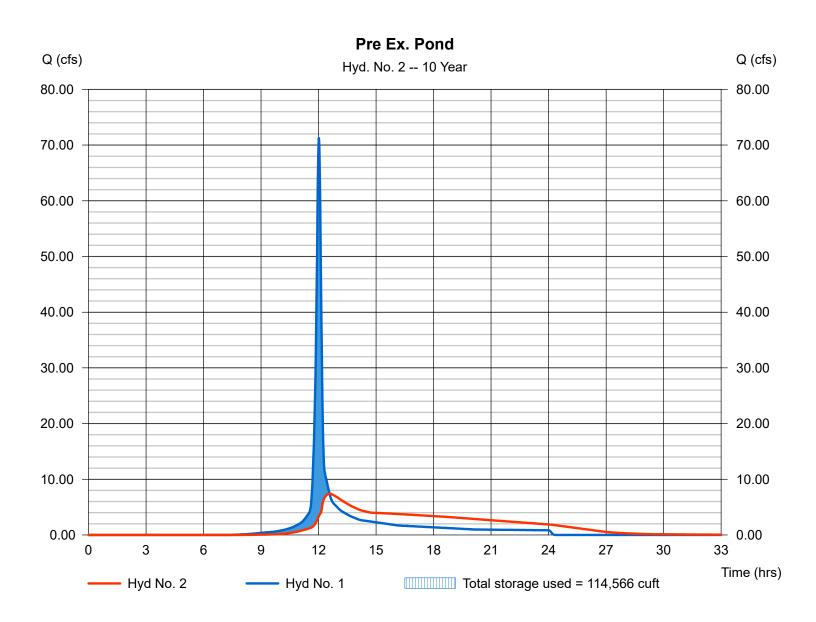
Monday, Jul 10, 2017

Hyd. No. 2

Pre Ex. Pond

Hydrograph type = Reservoir Peak discharge = 7.416 cfsStorm frequency Time to peak = 10 yrs $= 12.57 \, hrs$ Time interval = 1 min Hyd. volume = 180,534 cuftInflow hyd. No. = 1 - Pre Basin A2- to Ex. Detention Pond Max. Elevation = 988.83 ftReservoir name = Ex. Pond Max. Storage = 114,566 cuft

Storage Indication method used. Wet pond routing start elevation = 982.00 ft.



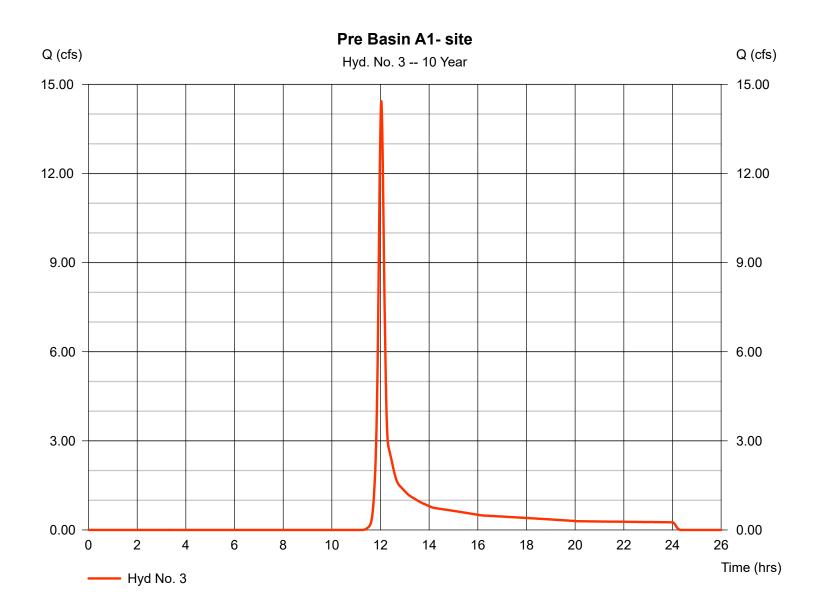
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Monday, Jul 10, 2017

Hyd. No. 3

Pre Basin A1- site

Hydrograph type = 14.44 cfs= SCS Runoff Peak discharge Storm frequency Time to peak = 10 yrs $= 12.03 \, hrs$ Time interval = 1 min Hyd. volume = 38,511 cuftDrainage area = 7.000 acCurve number = 59 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 12.80 \, \text{min}$ Distribution Total precip. = 5.52 in= Type II Storm duration = 484 = 24 hrs Shape factor



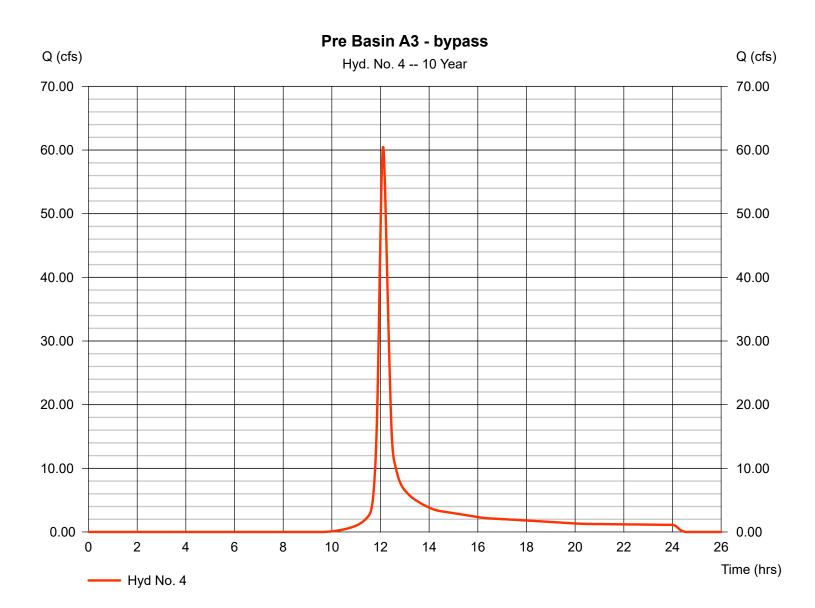
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Monday, Jul 10, 2017

Hyd. No. 4

Pre Basin A3 - bypass

Hydrograph type = SCS Runoff Peak discharge = 60.46 cfsStorm frequency Time to peak = 12.10 hrs= 10 yrsTime interval = 1 min Hyd. volume = 201,822 cuft Drainage area = 23.500 acCurve number = 69 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 21.80 \, \text{min}$ Distribution Total precip. = 5.52 in= Type II Storm duration = 24 hrs Shape factor = 484



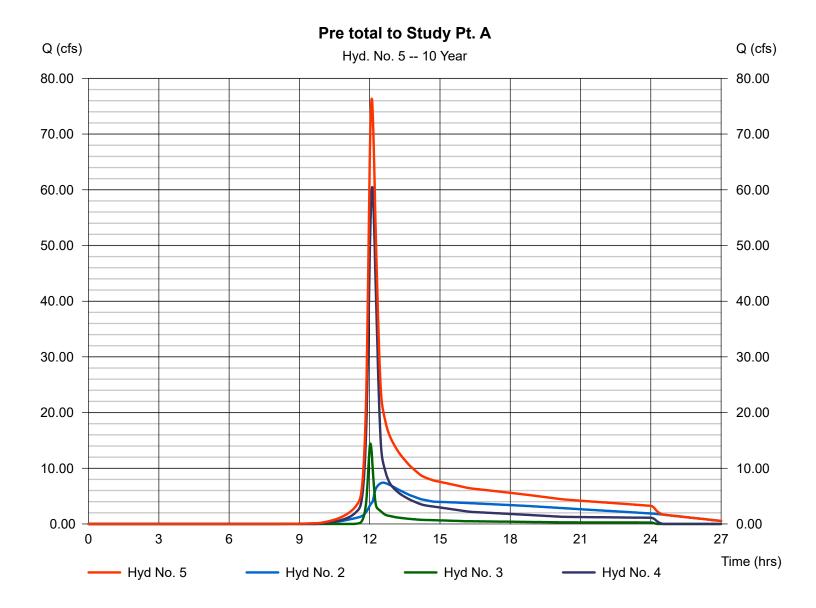
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Monday, Jul 10, 2017

Hyd. No. 5

Pre total to Study Pt. A

Hydrograph type = Combine Storm frequency = 10 yrs Time interval = 1 min Inflow hyds. = 2, 3, 4 Peak discharge = 76.36 cfs Time to peak = 12.08 hrs Hyd. volume = 420,868 cuft Contrib. drain. area = 30.500 ac



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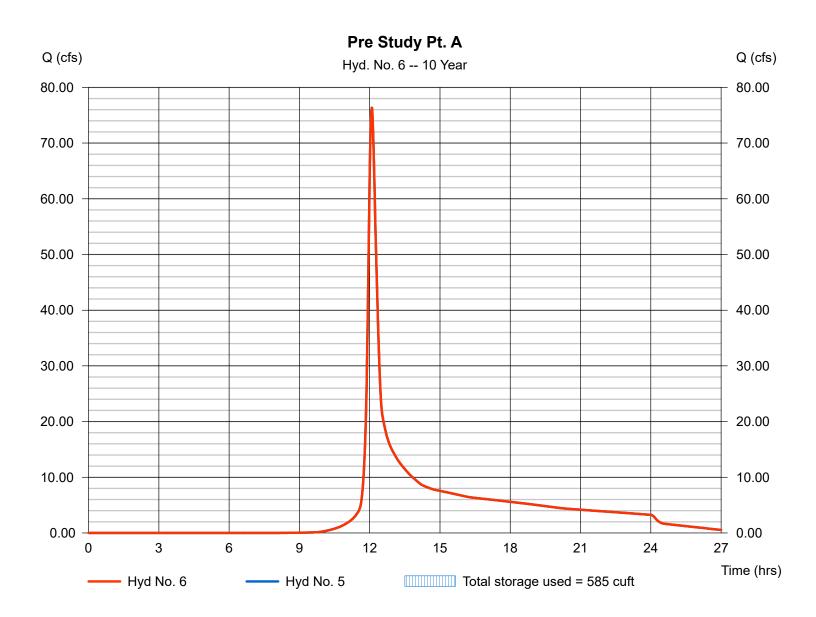
Monday, Jul 10, 2017

Hyd. No. 6

Pre Study Pt. A

Hydrograph type = Reservoir Peak discharge = 76.32 cfsStorm frequency Time to peak = 10 yrs= 12.10 hrsTime interval = 1 min Hyd. volume = 420,867 cuft Inflow hyd. No. = 5 - Pre total to Study Pt. A Max. Elevation = 972.04 ftReservoir name = Ex. DS Culvert at Barclay Max. Storage = 585 cuft

Storage Indication method used.



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= 61.94 cfs

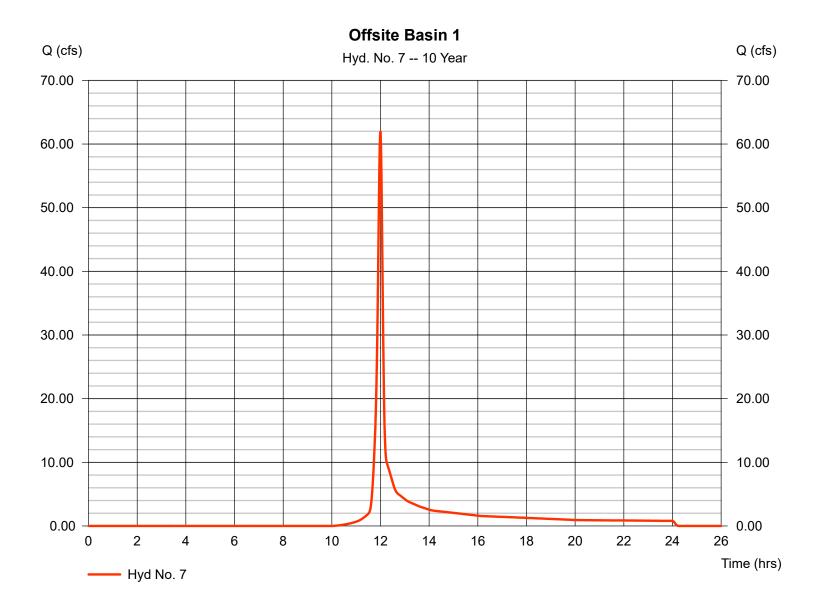
Hyd. No. 7

Offsite Basin 1

Hydrograph type = SCS Runoff Storm frequency = 10 yrsTime interval = 1 min Drainage area = 17.800 ac Basin Slope = 0.0 % Tc method = TR55 Total precip. = 5.52 inStorm duration = 24 hrs

Time to peak = 12.00 hrs
Hyd. volume = 140,463 cuft
Curve number = 67
Hydraulic length = 0 ft
Time of conc. (Tc) = 8.80 min
Distribution = Type II
Shape factor = 484

Peak discharge



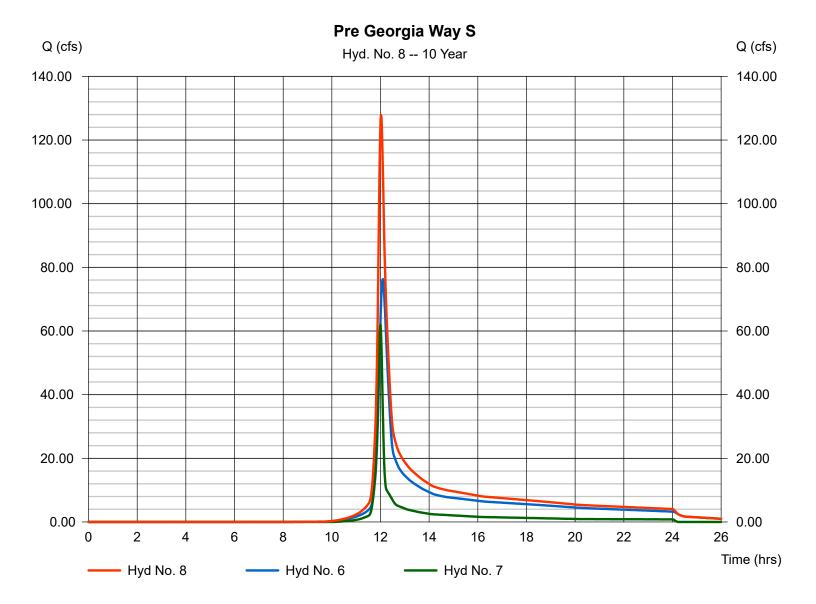
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Monday, Jul 10, 2017

Hyd. No. 8

Pre Georgia Way S

Hydrograph type = Combine Storm frequency = 10 yrs Time interval = 1 min Inflow hyds. = 6, 7 Peak discharge = 127.85 cfs Time to peak = 12.02 hrs Hyd. volume = 561,331 cuft Contrib. drain. area = 17.800 ac



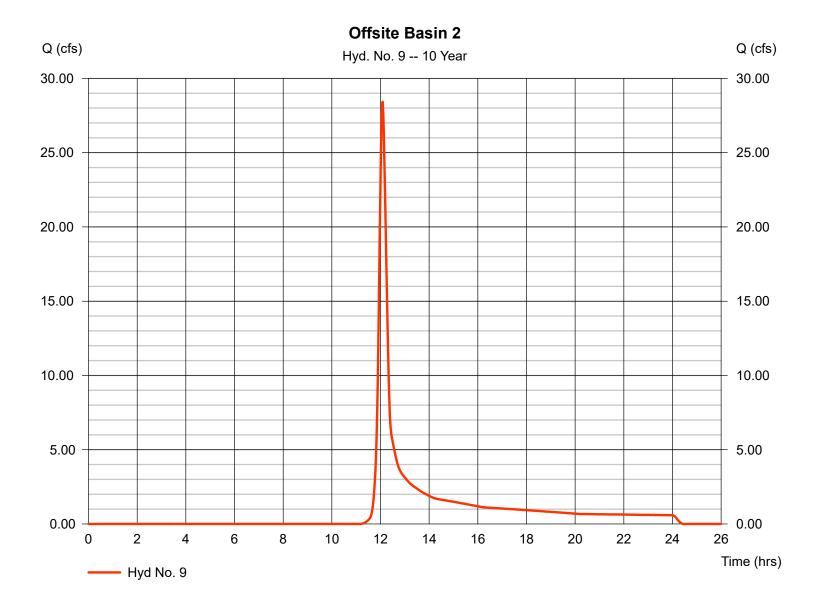
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Monday, Jul 10, 2017

Hyd. No. 9

Offsite Basin 2

Hydrograph type = SCS Runoff Peak discharge = 28.43 cfsStorm frequency Time to peak = 12.08 hrs= 10 yrsTime interval = 1 min Hyd. volume = 89,836 cuft Drainage area = 15.500 acCurve number = 60 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 17.30 minDistribution Total precip. = 5.52 in= Type II Storm duration = 24 hrs Shape factor = 484



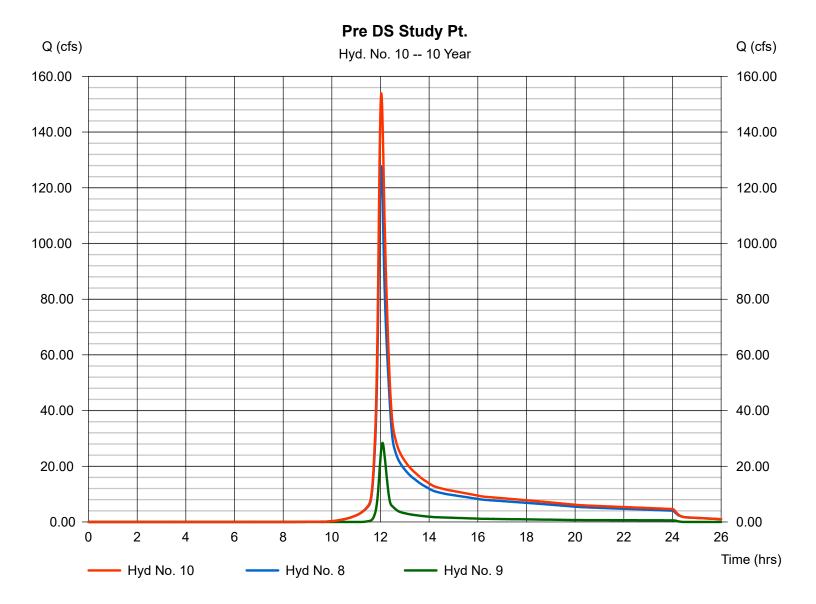
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Monday, Jul 10, 2017

Hyd. No. 10

Pre DS Study Pt.

Hydrograph type = Combine Storm frequency = 10 yrs Time interval = 1 min Inflow hyds. = 8, 9 Peak discharge = 153.93 cfs Time to peak = 12.03 hrs Hyd. volume = 651,166 cuft Contrib. drain. area = 15.500 ac



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Monday, Jul 10, 2017

Hyd. No. 12

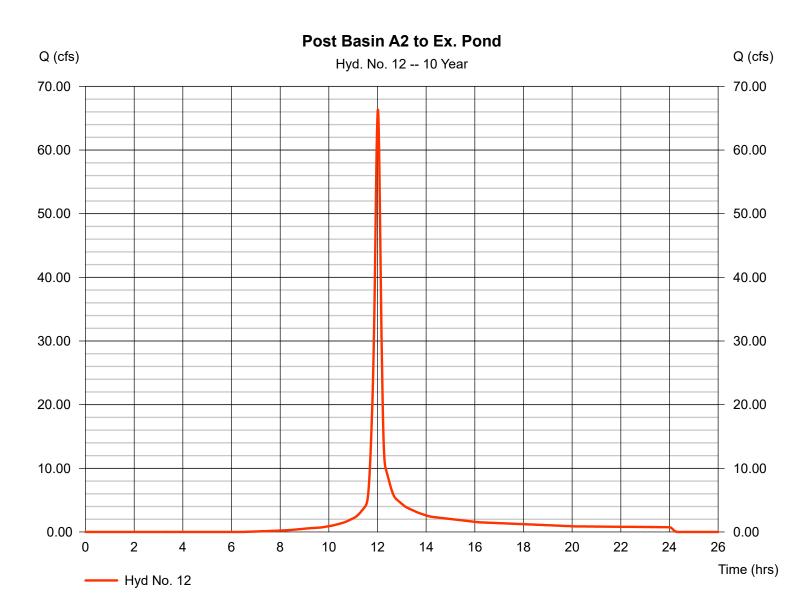
Post Basin A2 to Ex. Pond

Hydrograph type = SCS Runoff Storm frequency = 10 yrsTime interval = 1 min Drainage area = 13.730 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 5.52 inStorm duration = 24 hrs

Peak discharge = 66.33 cfs
Time to peak = 12.02 hrs
Hyd. volume = 169,216 cuft
Curve number = 81

Hydraulic length = 81 Time of conc. (Tc) = 12.4

Time of conc. (Tc) = 12.40 min
Distribution = Type II
Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

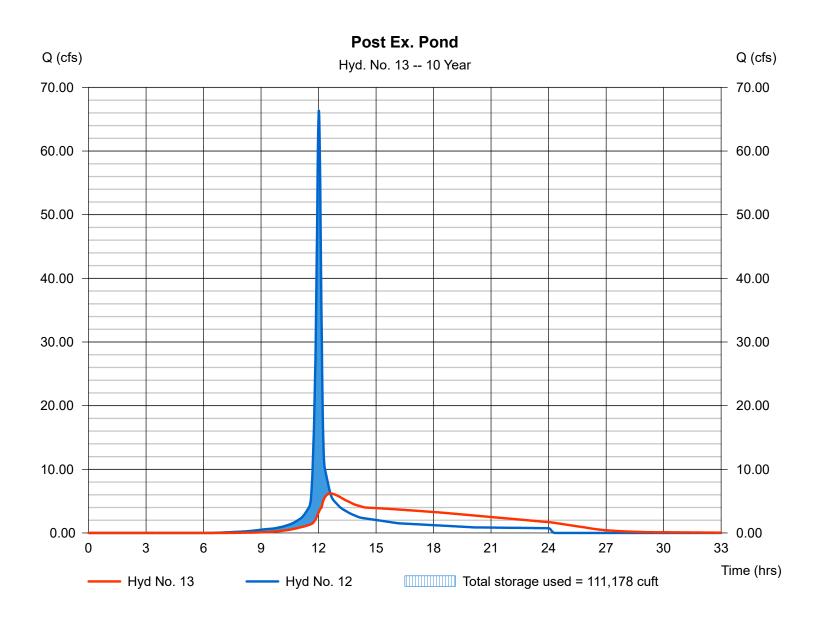
Monday, Jul 10, 2017

Hyd. No. 13

Post Ex. Pond

Hydrograph type = Reservoir Peak discharge = 6.232 cfsStorm frequency Time to peak = 10 yrs $= 12.62 \, hrs$ Time interval = 1 min Hyd. volume = 169,180 cuftInflow hyd. No. = 12 - Post Basin A2 to Ex. Pond Max. Elevation = 988.62 ftReservoir name = Ex. Pond Max. Storage = 111,178 cuft

Storage Indication method used. Wet pond routing start elevation = 982.00 ft.



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Hyd. No. 14

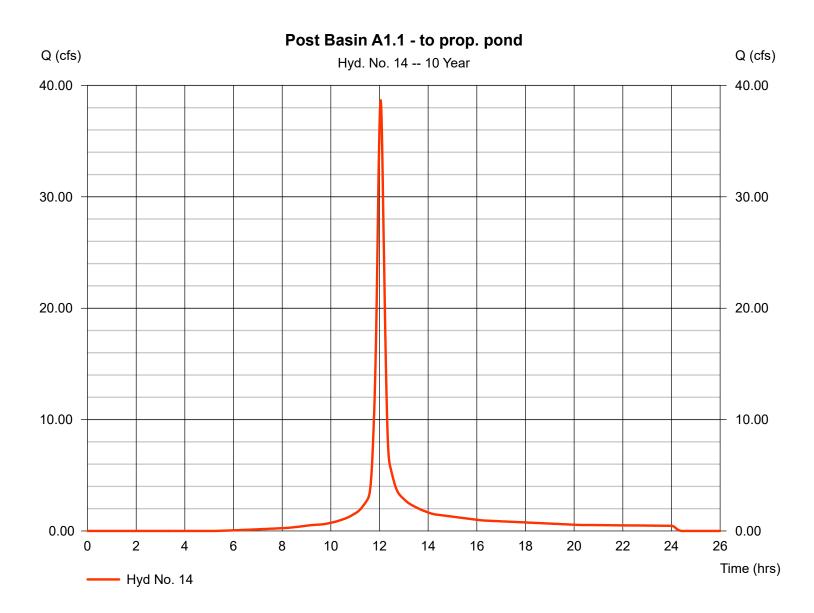
Post Basin A1.1 - to prop. pond

Hydrograph type = SCS Runoff Storm frequency = 10 yrsTime interval = 1 min Drainage area = 7.930 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 5.52 inStorm duration = 24 hrs

Peak discharge = 38.66 cfs
Time to peak = 12.05 hrs
Hyd. volume = 112,258 cuft
Curve number = 85

Curve number = 85 Hydraulic length = 0 ft

Time of conc. (Tc) = 16.30 min
Distribution = Type II
Shape factor = 484



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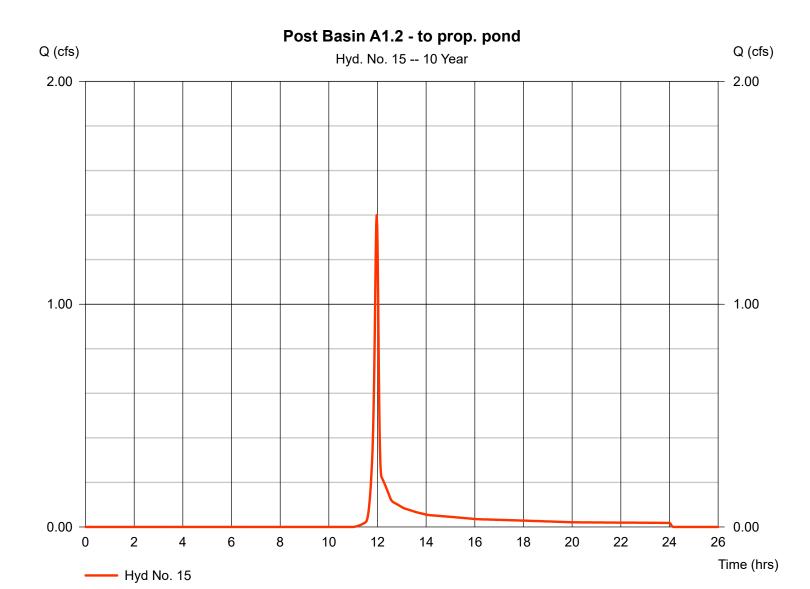
Monday, Jul 10, 2017

Hyd. No. 15

Post Basin A1.2 - to prop. pond

Hydrograph type = SCS Runoff Storm frequency = 10 yrsTime interval = 1 min Drainage area = 0.450 acBasin Slope = 0.0 % Tc method = USER Total precip. = 5.52 inStorm duration = 24 hrs

Peak discharge = 1.399 cfsTime to peak $= 11.97 \, hrs$ Hyd. volume = 2,849 cuftCurve number = 61 Hydraulic length = 0 ftTime of conc. (Tc) $= 5.00 \, \text{min}$ Distribution = Type II Shape factor = 484



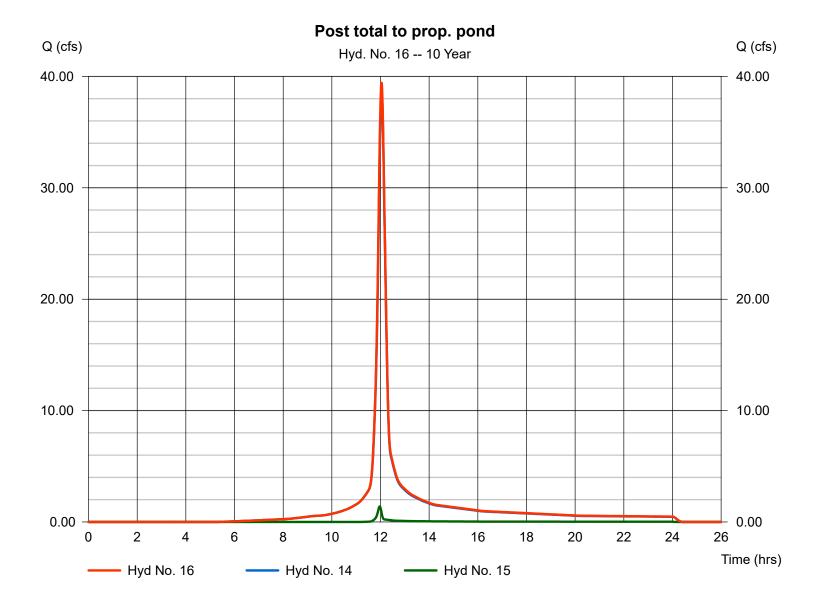
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Hyd. No. 16

Post total to prop. pond

Hydrograph type = Combine Storm frequency = 10 yrs Time interval = 1 min Inflow hyds. = 14, 15 Peak discharge = 39.42 cfs Time to peak = 12.03 hrs Hyd. volume = 115,108 cuft Contrib. drain. area = 8.380 ac



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

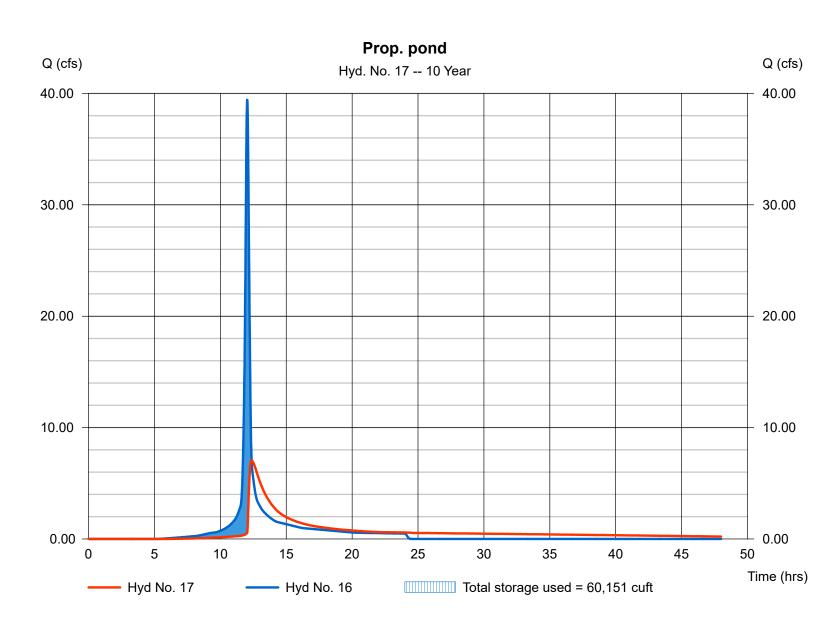
Monday, Jul 10, 2017

Hyd. No. 17

Prop. pond

Hydrograph type = Reservoir Peak discharge = 7.055 cfsStorm frequency Time to peak = 10 yrs $= 12.37 \, hrs$ Time interval = 1 min Hyd. volume = 108,861 cuft Inflow hyd. No. = 16 - Post total to prop. pond Max. Elevation $= 983.95 \, \mathrm{ft}$ Reservoir name = Stormwater Pond Max. Storage = 60,151 cuft

Storage Indication method used.



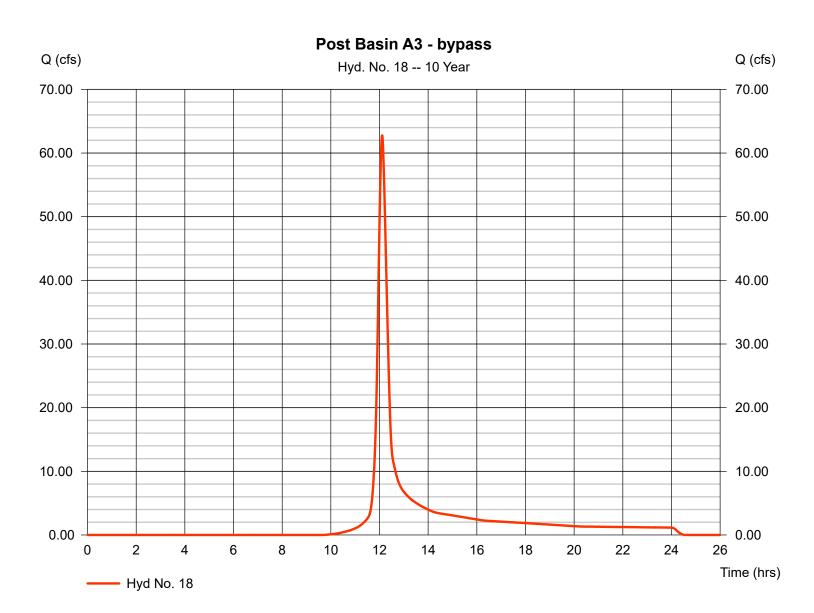
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 18

Post Basin A3 - bypass

Hydrograph type = SCS Runoff Peak discharge = 62.75 cfsStorm frequency Time to peak = 10 yrs= 12.10 hrsTime interval = 1 min Hyd. volume = 209,465 cuft Drainage area = 24.390 acCurve number = 69 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 21.80 \, \text{min}$ Distribution Total precip. = 5.52 in= Type II Storm duration = 24 hrs Shape factor = 484



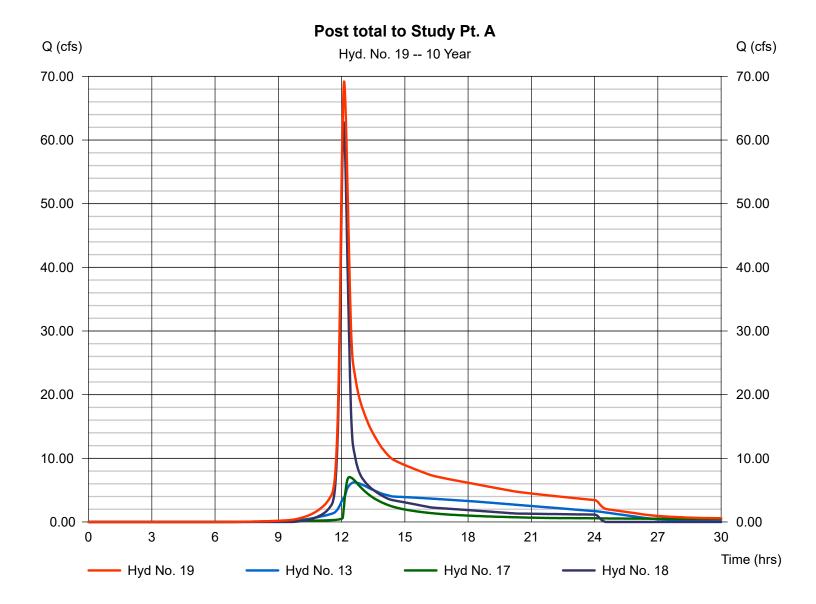
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 19

Post total to Study Pt. A

Hydrograph type = Combine Storm frequency = 10 yrs Time interval = 1 min Inflow hyds. = 13, 17, 18 Peak discharge = 69.21 cfs Time to peak = 12.12 hrs Hyd. volume = 487,506 cuft Contrib. drain. area = 24.390 ac



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

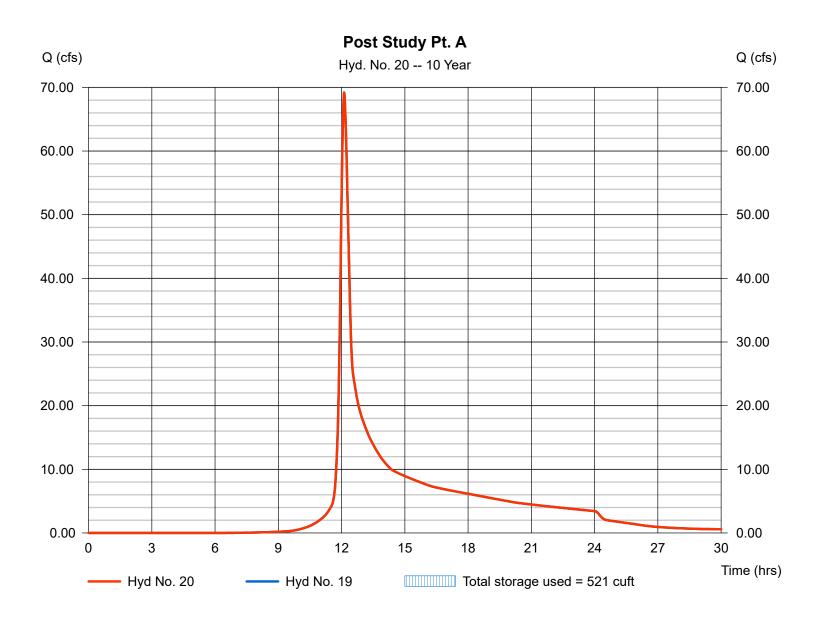
Monday, Jul 10, 2017

Hyd. No. 20

Post Study Pt. A

Hydrograph type = Reservoir Peak discharge = 69.17 cfsStorm frequency Time to peak = 10 yrs $= 12.12 \, hrs$ Time interval = 1 min Hyd. volume = 487,501 cuftInflow hyd. No. = 19 - Post total to Study Pt. A Max. Elevation = 971.84 ftReservoir name = Ex. DS Culvert at Barclay Max. Storage = 521 cuft

Storage Indication method used.



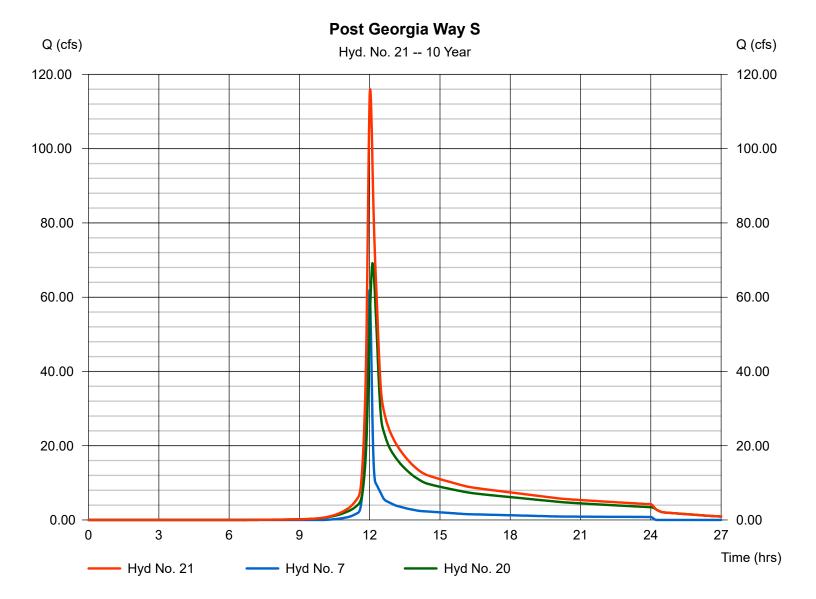
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 21

Post Georgia Way S

Hydrograph type = Combine Storm frequency = 10 yrs Time interval = 1 min Inflow hyds. = 7, 20 Peak discharge = 116.00 cfs Time to peak = 12.02 hrs Hyd. volume = 627,965 cuft Contrib. drain. area = 17.800 ac



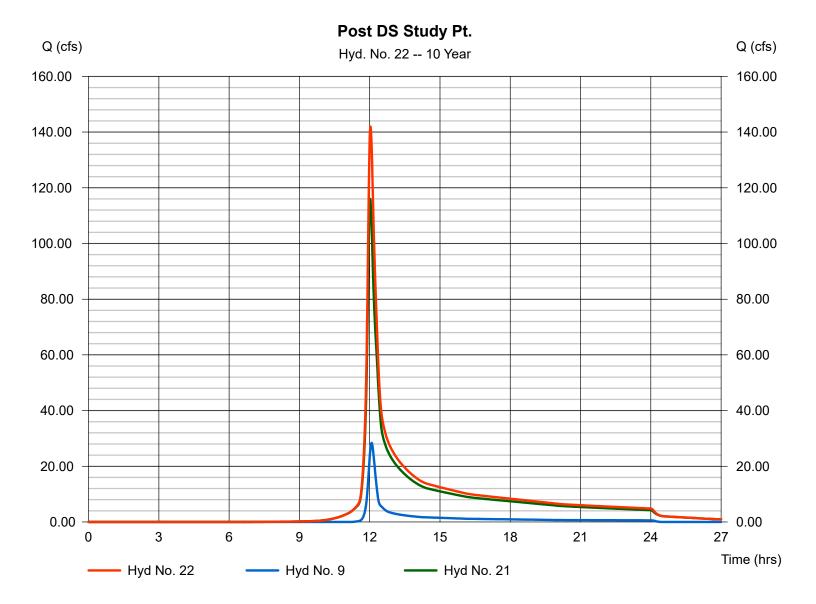
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 22

Post DS Study Pt.

Hydrograph type = Combine Storm frequency = 10 yrs Time interval = 1 min Inflow hyds. = 9, 21 Peak discharge = 142.00 cfs Time to peak = 12.03 hrs Hyd. volume = 717,801 cuft Contrib. drain. area = 15.500 ac



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

| Hyd. No. | Hydrograph type (origin) | Peak flow (cfs) | Time interval (min) | Time to peak (min) | Hyd. volume (cuft) | Inflow hyd(s) | Maximum elevation (ft) | Total strge used (cuft) | Hydrograph description | |
|-------------|--------------------------------|-----------------------|---------------------------|--------------------|--------------------------|------------------|------------------------------|-------------------------------|-------------------------------------|--|
| 1 | SCS Runoff | 89.87 | 1 | 721 | 229,032 | | | | Pre Basin A2- to Ex. Detention Pond | |
| 2 | Reservoir | 15.85 | 1 | 737 | 228,987 | 1 | 989.94 | 132,465 | Pre Ex. Pond | |
| 3 | SCS Runoff | 20.74 | 1 | 722 | 53,830 | | | | Pre Basin A1- site | |
| 4 | SCS Runoff | 80.62 | 1 | 726 | 266,326 | | | | Pre Basin A3 - bypass | |
| 5 | Combine | 106.07 | 1 | 726 | 549,143 | 2, 3, 4 | | | Pre total to Study Pt. A | |
| 6 | Reservoir | 105.91 | 1 | 727 | 549,143 | 5 | 972.91 | 1,191 | Pre Study Pt. A | |
| 7 | SCS Runoff | 82.75 | 1 | 720 | 187,228 | | | | Offsite Basin 1 | |
| 8 | Combine | 171.26 | 1 | 721 | 736,371 | 6, 7 | | | Pre Georgia Way S | |
| 9 | SCS Runoff | 40.67 | 1 | 725 | 124,725 | | | | Offsite Basin 2 | |
| 10 | Combine | 208.88 | 1 | 722 | 861,097 | 8, 9 | | | Pre DS Study Pt. | |
| 12 | SCS Runoff | 82.48 | 1 | 721 | 212,122 | | | | Post Basin A2 to Ex. Pond | |
| 13 | Reservoir | 13.03 | 1 | 738 | 212,080 | 12 | 989.61 | 127,201 | Post Ex. Pond | |
| 14 | SCS Runoff | 47.31 | 1 | 723 | 138,645 | | | | Post Basin A1.1 - to prop. pond | |
| 15 | SCS Runoff | 1.952 | 1 | 718 | 3,930 | | | | Post Basin A1.2 - to prop. pond | |
| 16 | Combine | 48.43 | 1 | 722 | 142,576 | 14, 15 | | | Post total to prop. pond | |
| 17 | Reservoir | 12.41 | 1 | 739 | 136,016 | 16 | 985.17 | 71,482 | Prop. pond | |
| 18 | SCS Runoff | 83.67 | 1 | 726 | 276,412 | | | | Post Basin A3 - bypass | |
| 19 | Combine | 99.53 | 1 | 728 | 624,509 | 13, 17, 18 | | | Post total to Study Pt. A | |
| 20 | Reservoir | 99.39 | 1 | 728 | 624,504 | 19 | 972.70 | 1,045 | Post Study Pt. A | |
| 21 | Combine | 155.65 | 1 | 721 | 811,731 | 7, 20 | | | Post Georgia Way S | |
| 22 | Combine | 192.97 | 1 | 722 | 936,456 | 9, 21 | | | Post DS Study Pt. | |
| 24 | Reservoir | 13.09 | 1 | 738 | 103,807 | 16 | 985.45 | 74,133 | Emergency Overflow | |
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07-11-17.gpw Return Period: 25 Year Monday, Jul 10, 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 1

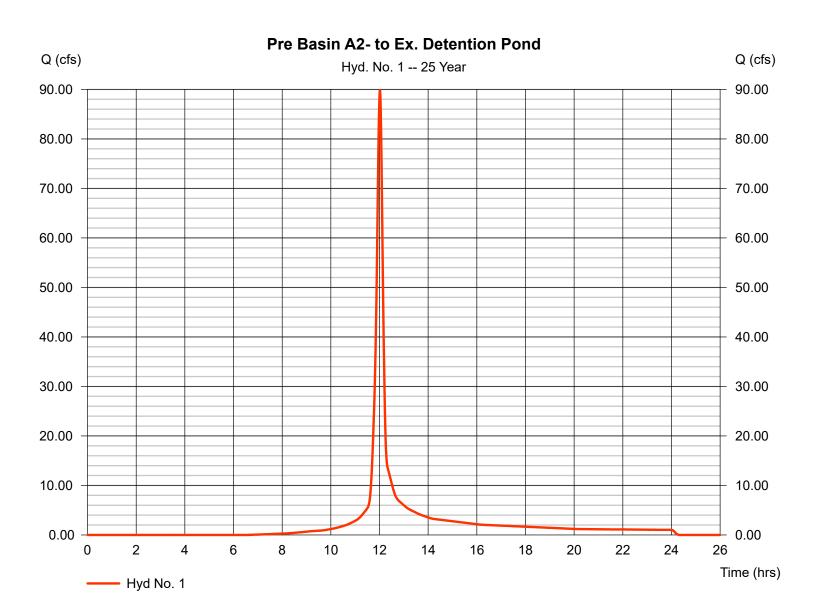
Pre Basin A2- to Ex. Detention Pond

= SCS Runoff Hydrograph type Storm frequency = 25 yrs Time interval = 1 min Drainage area = 16.000 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 6.48 inStorm duration = 24 hrs

Peak discharge = 89.87 cfs
Time to peak = 12.02 hrs
Hyd. volume = 229,032 cuft
Curve number = 78

Curve number = 78 Hydraulic length = 0 ft Time of conc. (Tc) = 12.40 min

Distribution = Type II Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

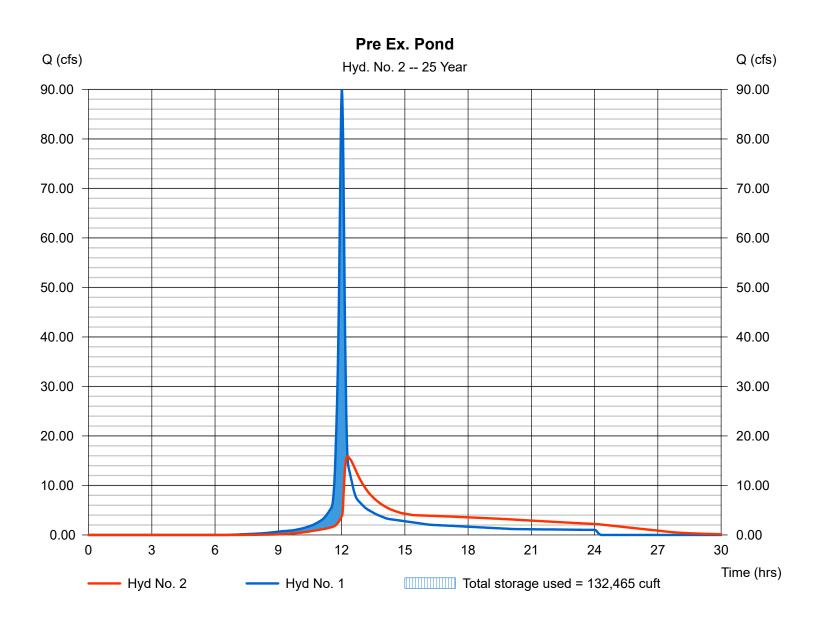
Monday, Jul 10, 2017

Hyd. No. 2

Pre Ex. Pond

Hydrograph type = Reservoir Peak discharge = 15.85 cfsStorm frequency Time to peak = 25 yrs $= 12.28 \, hrs$ Time interval = 1 min Hyd. volume = 228,987 cuft Inflow hyd. No. = 1 - Pre Basin A2- to Ex. Detention Pond Max. Elevation = 989.94 ftReservoir name = Ex. Pond Max. Storage = 132,465 cuft

Storage Indication method used. Wet pond routing start elevation = 982.00 ft.



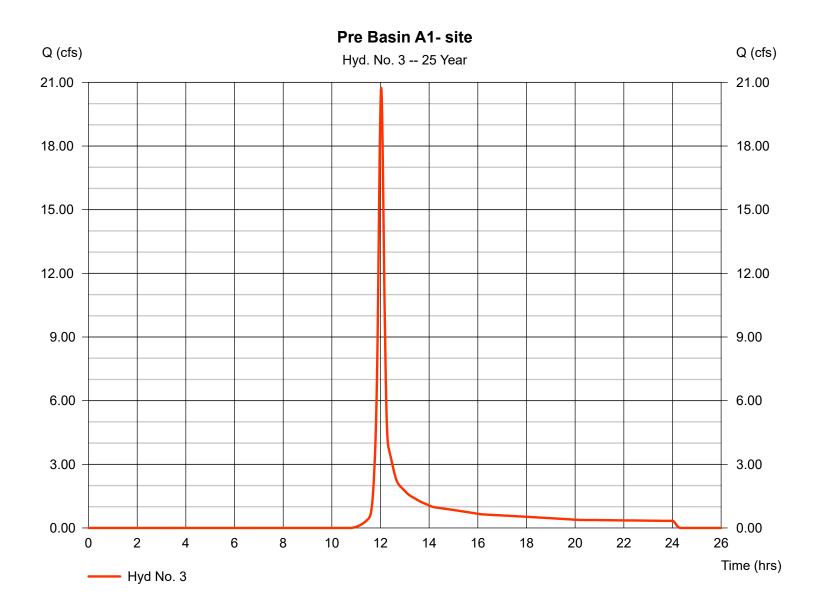
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 3

Pre Basin A1- site

Hydrograph type = SCS Runoff Peak discharge = 20.74 cfsStorm frequency Time to peak = 25 yrs $= 12.03 \, hrs$ Time interval = 1 min Hyd. volume = 53,830 cuftDrainage area = 7.000 acCurve number = 59 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 12.80 \, \text{min}$ Distribution Total precip. = 6.48 in= Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 4

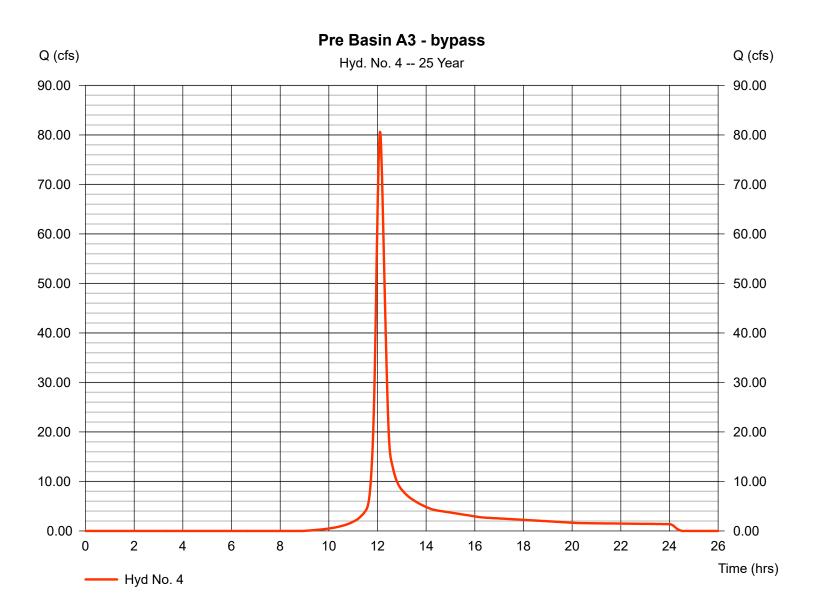
Pre Basin A3 - bypass

Hydrograph type = SCS Runoff Storm frequency = 25 yrs Time interval = 1 min Drainage area = 23.500 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 6.48 inStorm duration = 24 hrs

Peak discharge = 80.62 cfs
Time to peak = 12.10 hrs
Hyd. volume = 266,326 cuft

Curve number = 69 Hydraulic length = 0 ft

Time of conc. (Tc) = 21.80 min
Distribution = Type II
Shape factor = 484



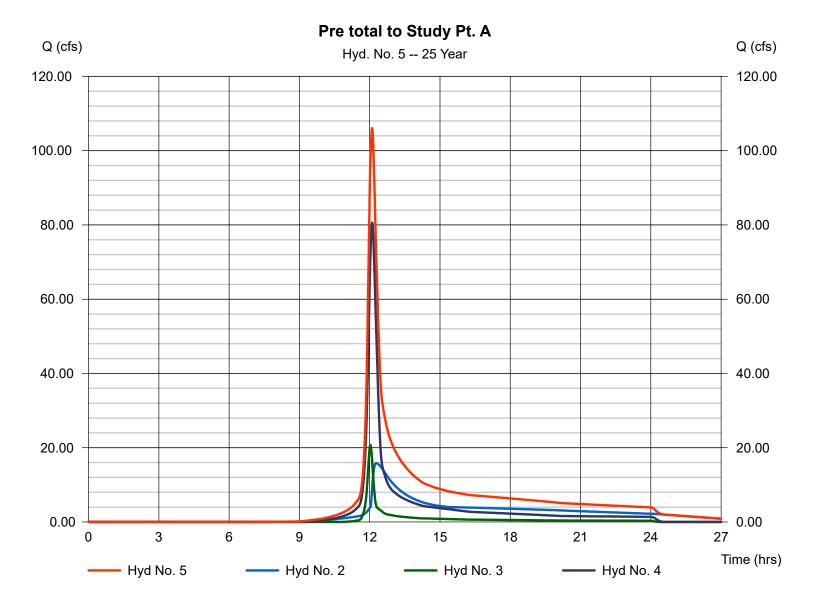
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 5

Pre total to Study Pt. A

Hydrograph type = Combine Storm frequency = 25 yrs Time interval = 1 min Inflow hyds. = 2, 3, 4 Peak discharge = 106.07 cfs Time to peak = 12.10 hrs Hyd. volume = 549,143 cuft Contrib. drain. area = 30.500 ac



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

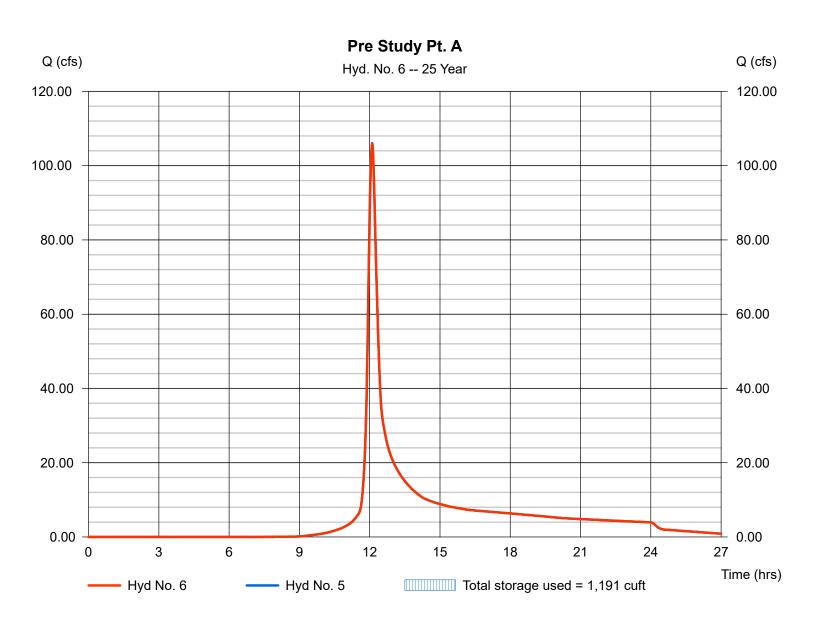
Monday, Jul 10, 2017

Hyd. No. 6

Pre Study Pt. A

Hydrograph type = Reservoir Peak discharge = 105.91 cfsStorm frequency Time to peak = 25 yrs $= 12.12 \, hrs$ Time interval = 1 min Hyd. volume = 549,143 cuft Inflow hyd. No. = 5 - Pre total to Study Pt. A Max. Elevation $= 972.91 \, \text{ft}$ Reservoir name = Ex. DS Culvert at Barclay Max. Storage = 1,191 cuft

Storage Indication method used.



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 7

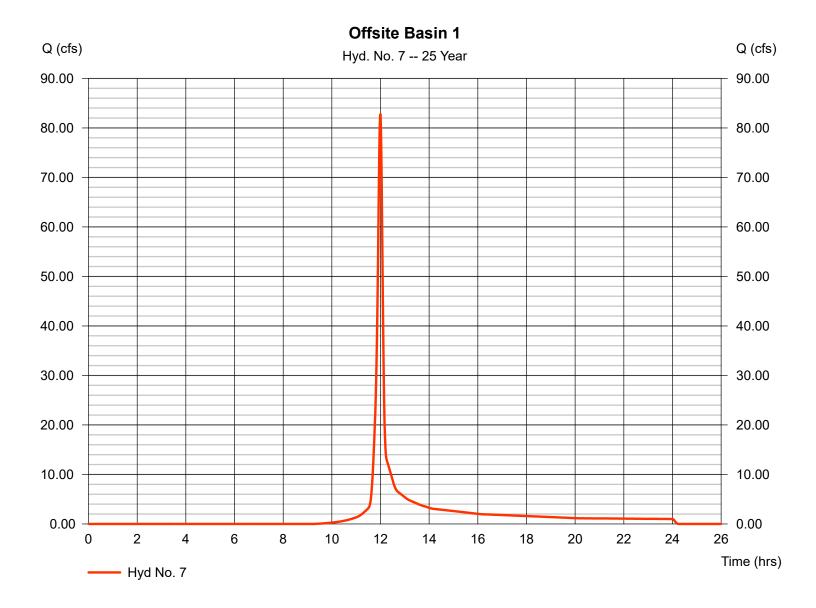
Offsite Basin 1

Hydrograph type = SCS Runoff Storm frequency = 25 yrs Time interval = 1 min Drainage area = 17.800 ac Basin Slope = 0.0 % Tc method = TR55 Total precip. = 6.48 inStorm duration = 24 hrs

Peak discharge = 82.75 cfs
Time to peak = 12.00 hrs
Hyd. volume = 187,228 cuft
Curve number = 67
Hydraulic length = 0 ft
Time of conc. (Tc) = 8.80 min
Distribution = Type II

= 484

Shape factor



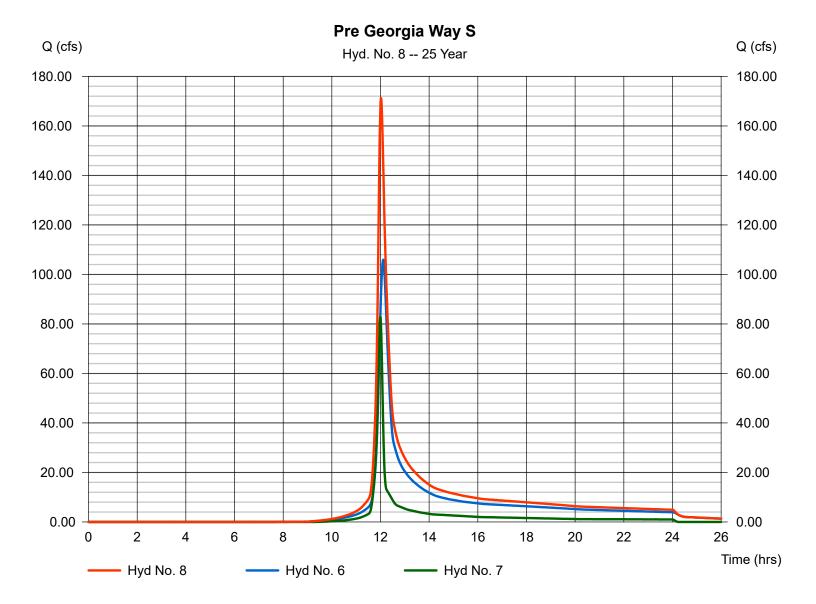
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 8

Pre Georgia Way S

Hydrograph type = Combine Storm frequency = 25 yrs Time interval = 1 min Inflow hyds. = 6, 7 Peak discharge = 171.26 cfs Time to peak = 12.02 hrs Hyd. volume = 736,371 cuft Contrib. drain. area = 17.800 ac



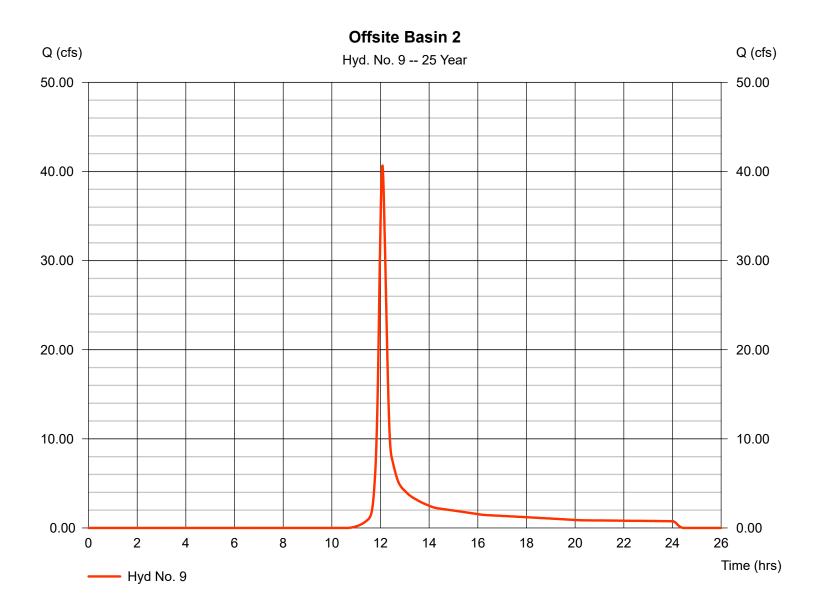
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 9

Offsite Basin 2

Hydrograph type = SCS Runoff Peak discharge = 40.67 cfsStorm frequency Time to peak = 25 yrs $= 12.08 \, hrs$ Time interval = 1 min Hyd. volume = 124,725 cuft Drainage area = 15.500 acCurve number = 60 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 17.30 minTotal precip. = 6.48 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



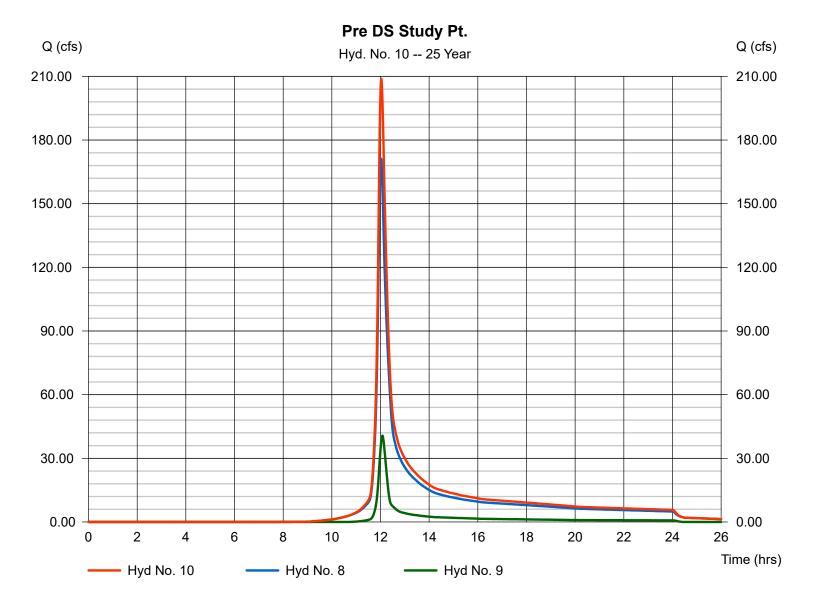
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 10

Pre DS Study Pt.

Hydrograph type = Combine Storm frequency = 25 yrs Time interval = 1 min Inflow hyds. = 8, 9 Peak discharge = 208.88 cfs Time to peak = 12.03 hrs Hyd. volume = 861,097 cuft Contrib. drain. area = 15.500 ac



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 12

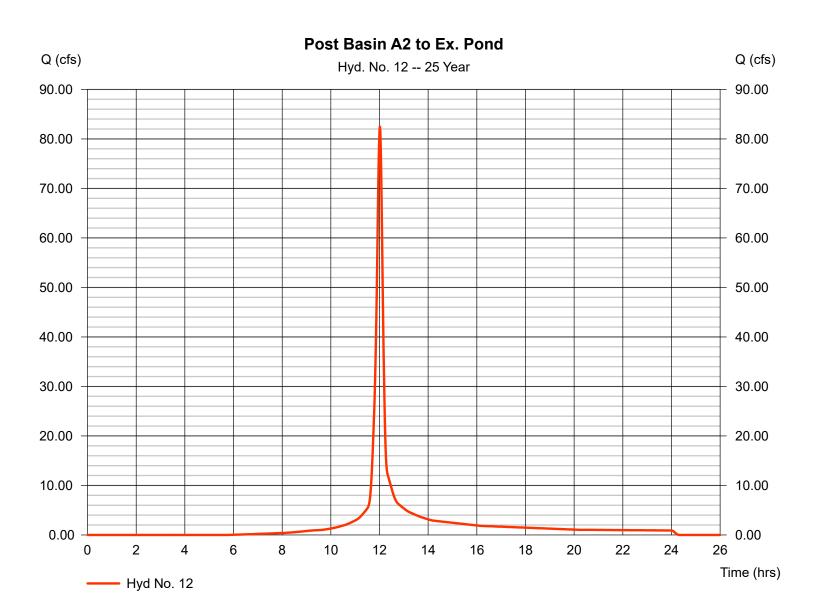
Post Basin A2 to Ex. Pond

Hydrograph type = SCS Runoff Storm frequency = 25 yrs Time interval = 1 min Drainage area = 13.730 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 6.48 inStorm duration = 24 hrs

Peak discharge = 82.48 cfs
Time to peak = 12.02 hrs
Hyd. volume = 212,122 cuft

Curve number = 81 Hydraulic length = 0 ft

Time of conc. (Tc) = 12.40 min
Distribution = Type II
Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

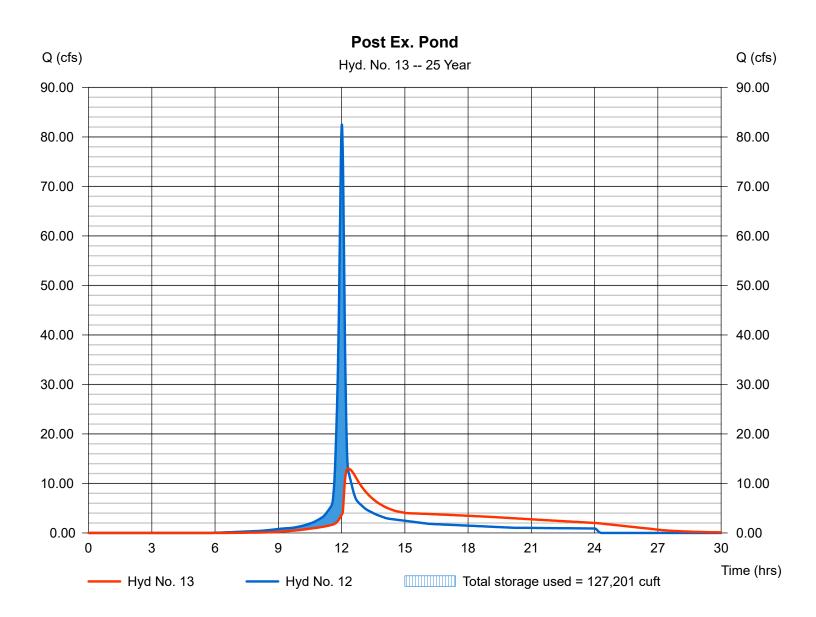
Monday, Jul 10, 2017

Hyd. No. 13

Post Ex. Pond

Hydrograph type = Reservoir Peak discharge = 13.03 cfsStorm frequency Time to peak = 25 yrs $= 12.30 \, hrs$ Time interval = 1 min Hyd. volume = 212,080 cuftInflow hyd. No. = 12 - Post Basin A2 to Ex. Pond Max. Elevation $= 989.61 \, \text{ft}$ Reservoir name = Ex. Pond Max. Storage = 127,201 cuft

Storage Indication method used. Wet pond routing start elevation = 982.00 ft.



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 14

Post Basin A1.1 - to prop. pond

Hydrograph type = SCS Runoff Storm frequency = 25 yrs Time interval = 1 min Drainage area = 7.930 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 6.48 inStorm duration = 24 hrs

Peak discharge = 47.31 cfs
Time to peak = 12.05 hrs
Hyd. volume = 138,645 cuft
Curve number = 85

Hydraulic length = 0 ft
Time of conc. (Tc) = 16.30 min
Distribution = Type II

= 484

Shape factor

Post Basin A1.1 - to prop. pond Q (cfs) Q (cfs) Hyd. No. 14 -- 25 Year 50.00 50.00 40.00 40.00 30.00 30.00 20.00 20.00 10.00 10.00 0.00 0.00 2 4 6 8 10 12 14 16 18 20 22 24 26 Time (hrs) Hyd No. 14

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

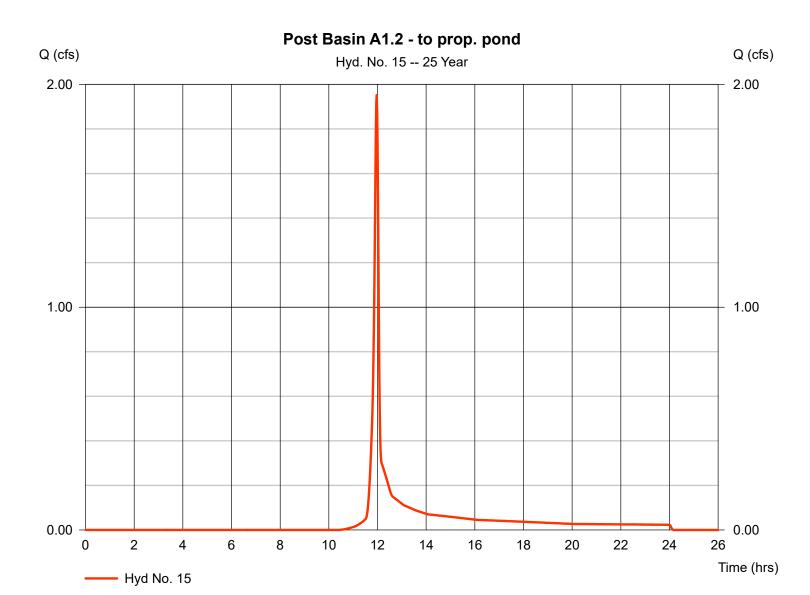
Monday, Jul 10, 2017

Hyd. No. 15

Post Basin A1.2 - to prop. pond

Hydrograph type = SCS Runoff Storm frequency = 25 yrs Time interval = 1 min Drainage area = 0.450 acBasin Slope = 0.0 % Tc method = USER Total precip. = 6.48 inStorm duration = 24 hrs

Peak discharge = 1.952 cfsTime to peak $= 11.97 \, hrs$ Hyd. volume = 3,930 cuftCurve number = 61 Hydraulic length = 0 ftTime of conc. (Tc) $= 5.00 \, \text{min}$ Distribution = Type II Shape factor = 484



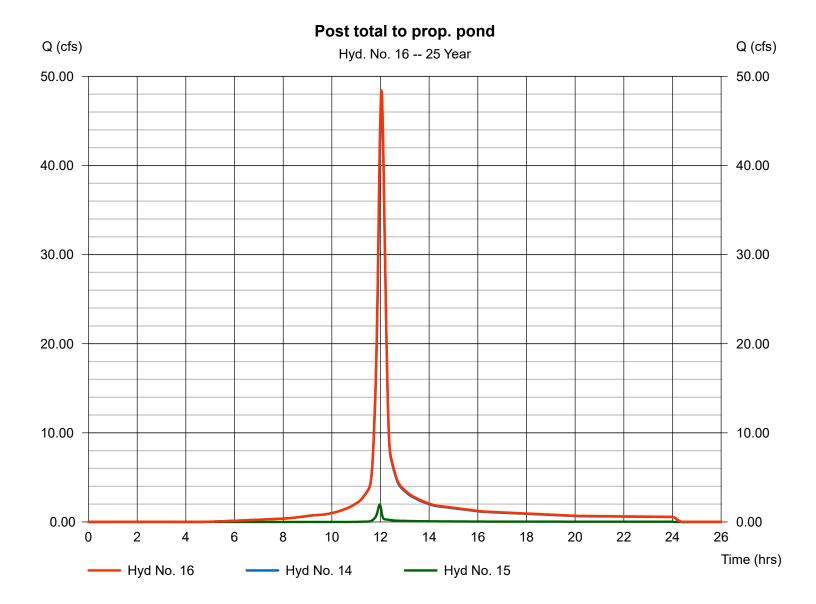
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 16

Post total to prop. pond

Hydrograph type = Combine Storm frequency = 25 yrs Time interval = 1 min Inflow hyds. = 14, 15 Peak discharge = 48.43 cfs Time to peak = 12.03 hrs Hyd. volume = 142,576 cuft Contrib. drain. area = 8.380 ac



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

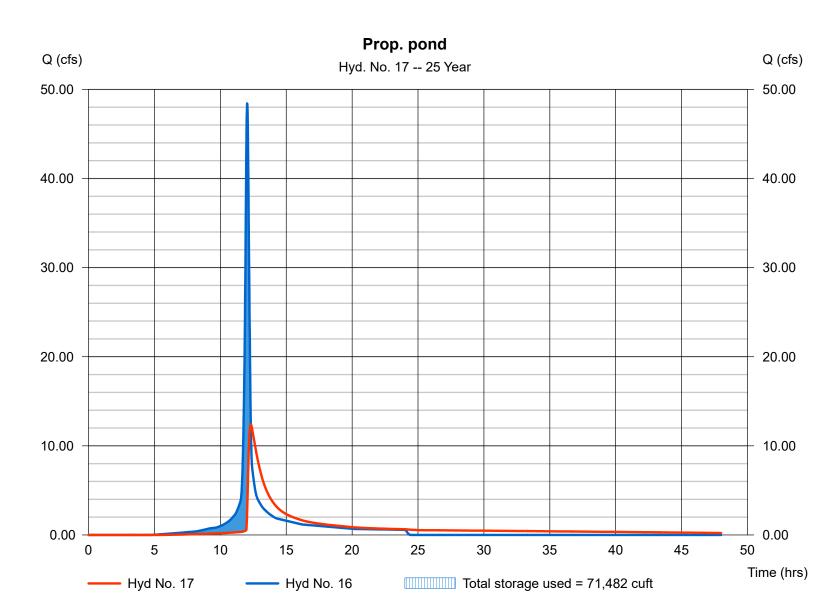
Monday, Jul 10, 2017

Hyd. No. 17

Prop. pond

Hydrograph type = Reservoir Peak discharge = 12.41 cfsStorm frequency Time to peak = 25 yrs $= 12.32 \, hrs$ Time interval = 1 min Hyd. volume = 136,016 cuft Inflow hyd. No. = 16 - Post total to prop. pond Max. Elevation $= 985.17 \, \mathrm{ft}$ Reservoir name = Stormwater Pond Max. Storage = 71,482 cuft

Storage Indication method used.



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

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Hyd. No. 18

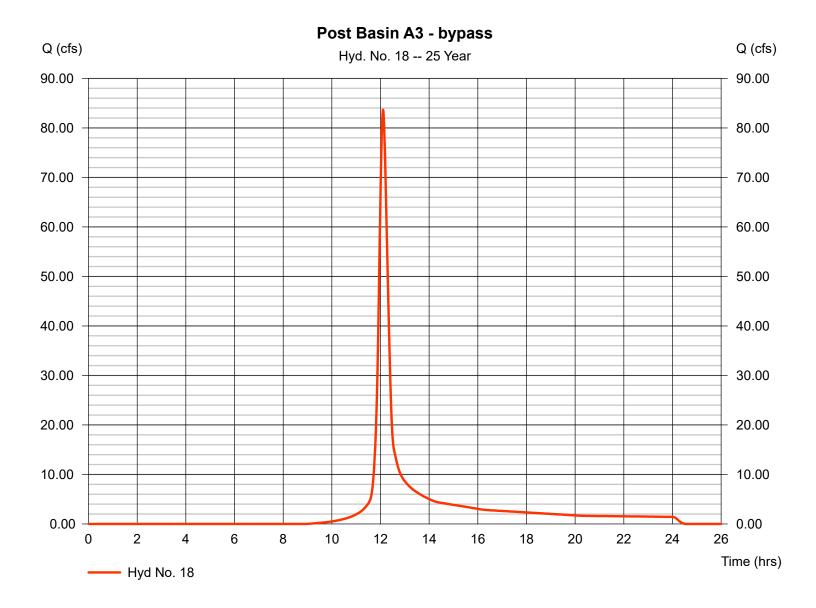
Post Basin A3 - bypass

Hydrograph type = SCS Runoff Storm frequency = 25 yrs Time interval = 1 min Drainage area = 24.390 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 6.48 inStorm duration = 24 hrs

Peak discharge = 83.67 cfs
Time to peak = 12.10 hrs
Hyd. volume = 276,412 cuft

Curve number = 69 Hydraulic length = 0 ft

Time of conc. (Tc) = 21.80 min
Distribution = Type II
Shape factor = 484



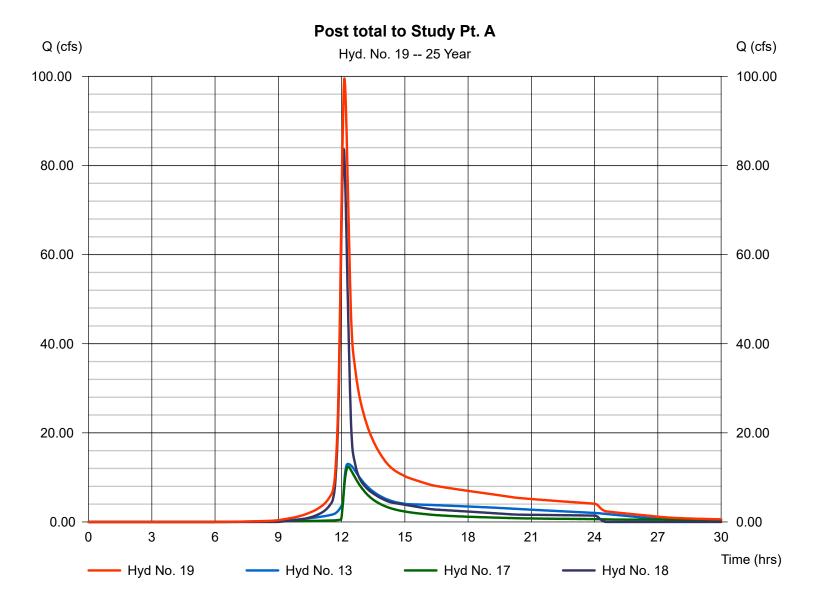
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 19

Post total to Study Pt. A

Hydrograph type = Combine Storm frequency = 25 yrs Time interval = 1 min Inflow hyds. = 13, 17, 18 Peak discharge = 99.53 cfs
Time to peak = 12.13 hrs
Hyd. volume = 624,509 cuft
Contrib. drain. area = 24.390 ac



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

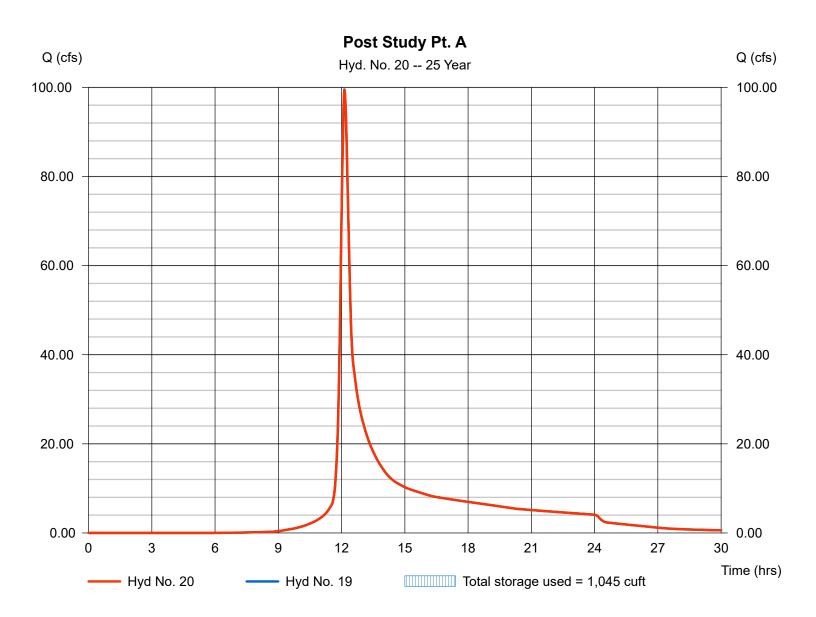
Monday, Jul 10, 2017

Hyd. No. 20

Post Study Pt. A

Hydrograph type = Reservoir Peak discharge = 99.39 cfsTime to peak Storm frequency = 25 yrs $= 12.13 \, hrs$ Time interval = 1 min Hyd. volume = 624,504 cuftInflow hyd. No. = 19 - Post total to Study Pt. A Max. Elevation = 972.70 ftReservoir name = Ex. DS Culvert at Barclay Max. Storage = 1,045 cuft

Storage Indication method used.



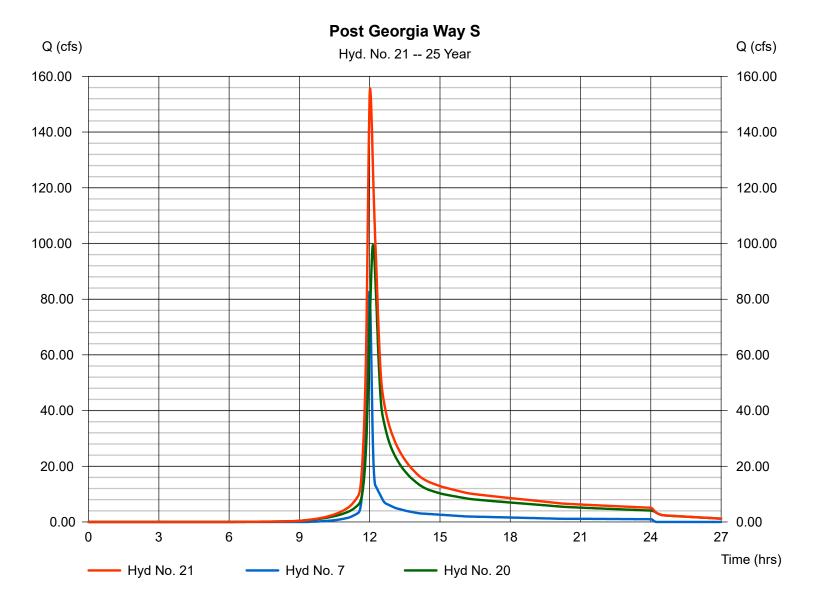
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 21

Post Georgia Way S

Hydrograph type = Combine Storm frequency = 25 yrs Time interval = 1 min Inflow hyds. = 7, 20 Peak discharge = 155.65 cfs Time to peak = 12.02 hrs Hyd. volume = 811,731 cuft Contrib. drain. area = 17.800 ac



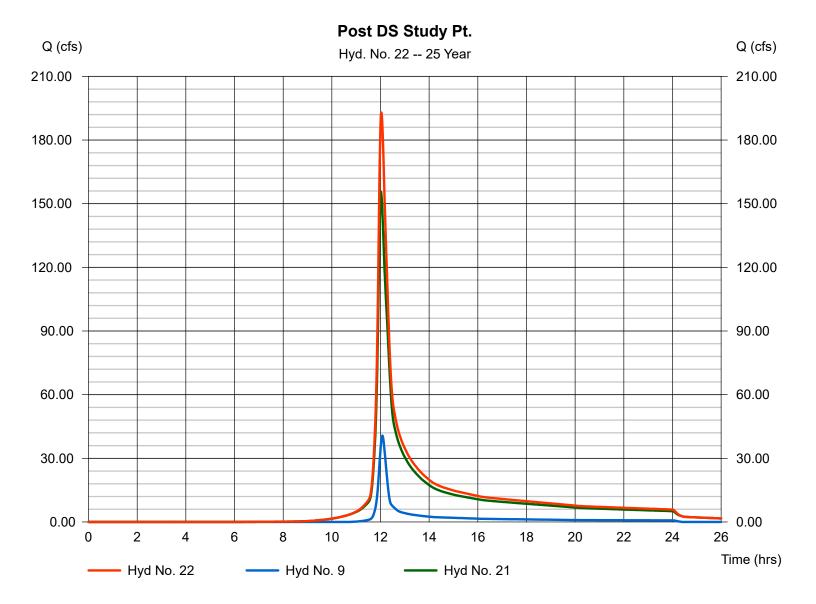
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 22

Post DS Study Pt.

Hydrograph type = Combine Storm frequency = 25 yrs Time interval = 1 min Inflow hyds. = 9, 21 Peak discharge = 192.97 cfs Time to peak = 12.03 hrs Hyd. volume = 936,456 cuft Contrib. drain. area = 15.500 ac



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

| Hyd. No. | Hydrograph type (origin) | Peak flow (cfs) | Time interval (min) | Time to peak (min) | Hyd. volume (cuft) | Inflow hyd(s) | Maximum elevation (ft) | Total strge used (cuft) | Hydrograph description | |
|-------------|--------------------------------|-----------------------|---------------------------|--------------------|--------------------------|------------------|------------------------------|-------------------------------|-------------------------------------|--|
| 1 | SCS Runoff | 103.95 | 1 | 721 | 266,232 | | | | Pre Basin A2- to Ex. Detention Pond | |
| 2 | Reservoir | 23.54 | 1 | 735 | 266,182 | 1 | 990.72 | 146,748 | Pre Ex. Pond | |
| 3 | SCS Runoff | 25.76 | 1 | 722 | 66,178 | | | | Pre Basin A1- site | |
| 4 | SCS Runoff | 96.26 | 1 | 726 | 316,826 | | | | Pre Basin A3 - bypass | |
| 5 | Combine | 133.27 | 1 | 726 | 649,186 | 2, 3, 4 | | | Pre total to Study Pt. A | |
| 6 | Reservoir | 132.72 | 1 | 727 | 649,185 | 5 | 974.01 | 1,993 | Pre Study Pt. A | |
| 7 | SCS Runoff | 99.03 | 1 | 719 | 224,028 | | | | Offsite Basin 1 | |
| 8 | Combine | 205.95 | 1 | 721 | 873,214 | 6, 7 | | | Pre Georgia Way S | |
| 9 | SCS Runoff | 50.41 | 1 | 725 | 152,754 | | | | Offsite Basin 2 | |
| 10 | Combine | 253.81 | 1 | 722 | 1,025,967 | 8, 9 | | | Pre DS Study Pt. | |
| 12 | SCS Runoff | 94.64 | 1 | 721 | 244,883 | | | | Post Basin A2 to Ex. Pond | |
| 13 | Reservoir | 19.74 | 1 | 735 | 244,838 | 12 | 990.35 | 139,918 | Post Ex. Pond | |
| 14 | SCS Runoff | 53.79 | 1 | 723 | 158,668 | | | | Post Basin A1.1 - to prop. pond | |
| 15 | SCS Runoff | 2.388 | 1 | 718 | 4,796 | | | | Post Basin A1.2 - to prop. pond | |
| 16 | Combine | 55.19 | 1 | 722 | 163,465 | 14, 15 | | | Post total to prop. pond | |
| 17 | Reservoir | 17.47 | 1 | 737 | 156,740 | 16 | 986.04 | 79,614 | Prop. pond | |
| 18 | SCS Runoff | 99.91 | 1 | 726 | 328,825 | | | | Post Basin A3 - bypass | |
| 19 | Combine | 126.62 | 1 | 728 | 730,403 | 13, 17, 18 | | | Post total to Study Pt. A | |
| 20 | Reservoir | 126.38 | 1 | 729 | 730,397 | 19 | 973.75 | 1,775 | Post Study Pt. A | |
| 21 | Combine | 187.86 | 1 | 722 | 954,425 | 7, 20 | | | Post Georgia Way S | |
| 22 | Combine | 235.96 | 1 | 722 | 1,107,179 | 9, 21 | | | Post DS Study Pt. | |
| 24 | Reservoir | 21.59 | 1 | 735 | 124,693 | 16 | 986.23 | 81,585 | Emergency Overflow | |
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07-11-17.gpw Return Period: 50 Year Monday, Jul 10, 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 1

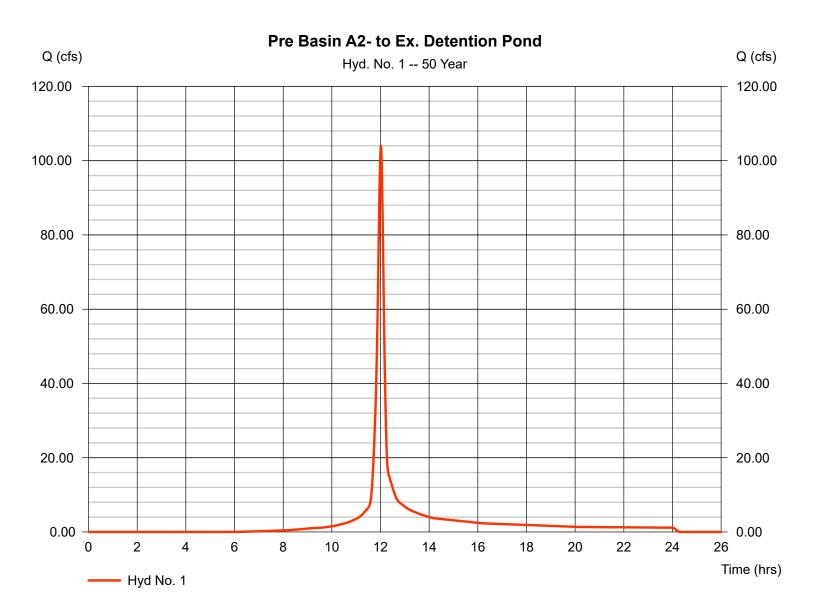
Pre Basin A2- to Ex. Detention Pond

= SCS Runoff Hydrograph type Storm frequency = 50 yrsTime interval = 1 min Drainage area = 16.000 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 7.20 inStorm duration = 24 hrs

Peak discharge = 103.95 cfs
Time to peak = 12.02 hrs
Hyd. volume = 266,232 cuft
Curve number = 78

Hydraulic length = 0 ft Time of conc. (Tc) = 12.4

Time of conc. (Tc) = 12.40 min
Distribution = Type II
Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

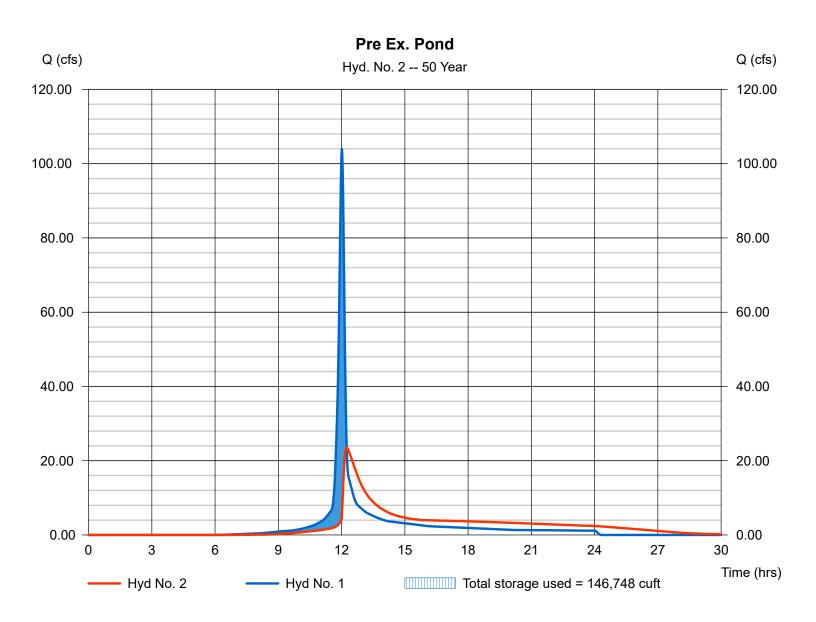
Monday, Jul 10, 2017

Hyd. No. 2

Pre Ex. Pond

Hydrograph type = Reservoir Peak discharge = 23.54 cfsStorm frequency Time to peak = 50 yrs $= 12.25 \, hrs$ Time interval = 1 min Hyd. volume = 266,182 cuft Inflow hyd. No. = 1 - Pre Basin A2- to Ex. Detention Pond Max. Elevation = 990.72 ftReservoir name = Ex. Pond Max. Storage = 146,748 cuft

Storage Indication method used. Wet pond routing start elevation = 982.00 ft.



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 3

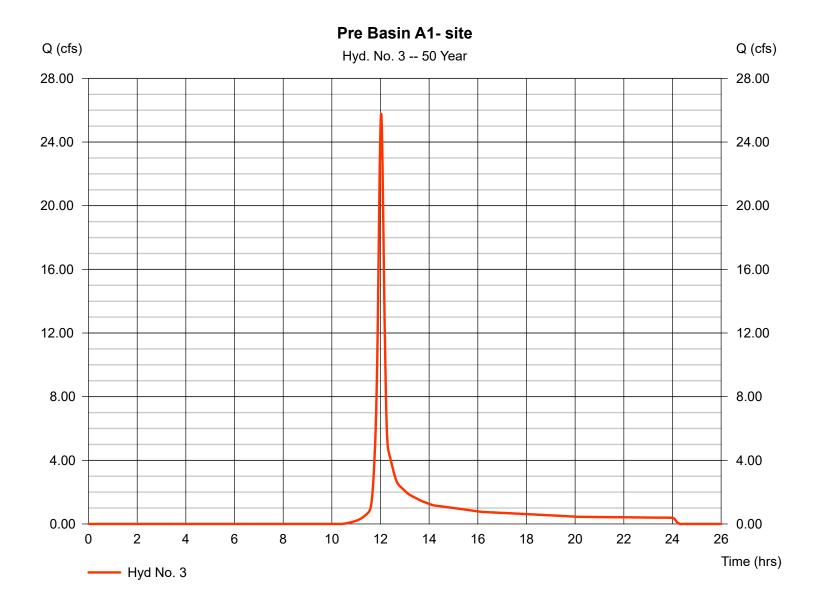
Pre Basin A1- site

Hydrograph type = SCS Runoff Storm frequency = 50 yrsTime interval = 1 min Drainage area = 7.000 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 7.20 inStorm duration = 24 hrs

Peak discharge = 25.76 cfs
Time to peak = 12.03 hrs
Hyd. volume = 66,178 cuft
Curve number = 59

Curve number = 59Hydraulic length = 0 ft

Time of conc. (Tc) = 12.80 min
Distribution = Type II
Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 4

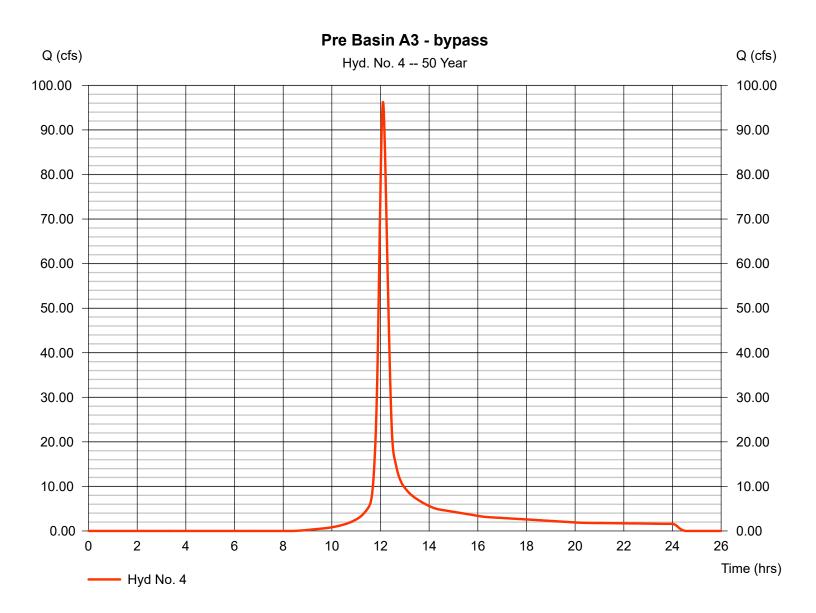
Pre Basin A3 - bypass

Hydrograph type = SCS Runoff Storm frequency = 50 yrsTime interval = 1 min Drainage area = 23.500 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 7.20 inStorm duration = 24 hrs

Peak discharge = 96.26 cfs
Time to peak = 12.10 hrs
Hyd. volume = 316,826 cuft

Curve number = 69 Hydraulic length = 0 ft

Time of conc. (Tc) = 21.80 min
Distribution = Type II
Shape factor = 484



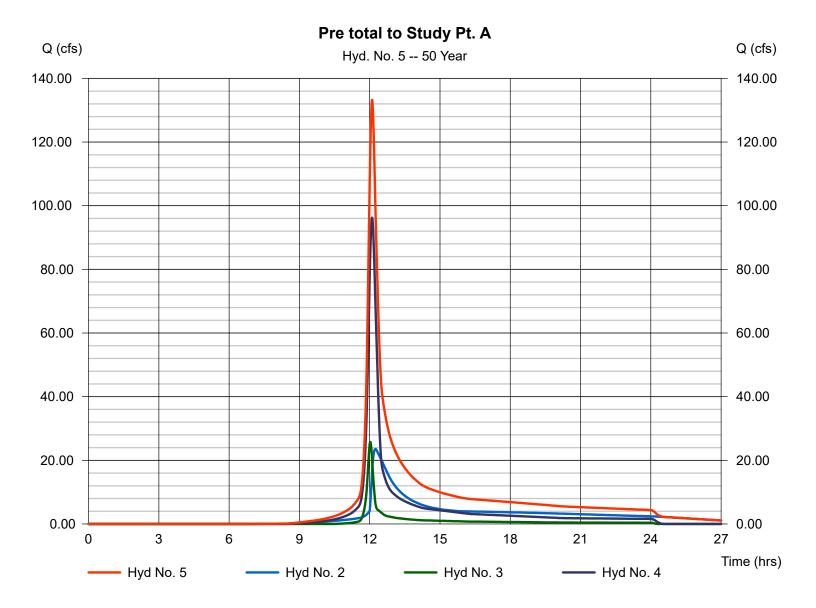
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 5

Pre total to Study Pt. A

Hydrograph type = Combine Storm frequency = 50 yrs Time interval = 1 min Inflow hyds. = 2, 3, 4 Peak discharge = 133.27 cfs Time to peak = 12.10 hrs Hyd. volume = 649,186 cuft Contrib. drain. area = 30.500 ac



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

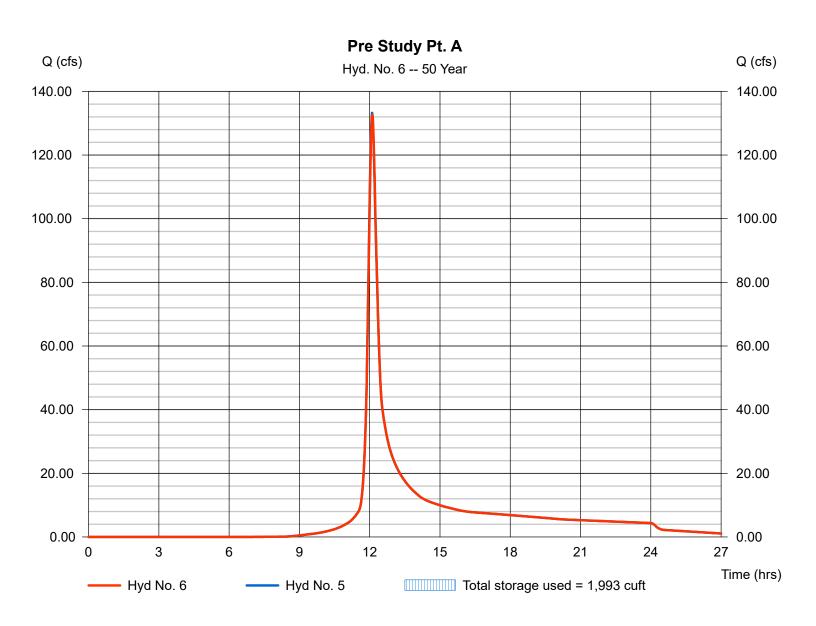
Monday, Jul 10, 2017

Hyd. No. 6

Pre Study Pt. A

Hydrograph type = Reservoir Peak discharge = 132.72 cfsStorm frequency Time to peak = 50 yrs $= 12.12 \, hrs$ Time interval = 1 min Hyd. volume = 649,185 cuft Inflow hyd. No. = 5 - Pre total to Study Pt. A Max. Elevation $= 974.01 \, \text{ft}$ Reservoir name = Ex. DS Culvert at Barclay Max. Storage = 1,993 cuft

Storage Indication method used.



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Monday, Jul 10, 2017

Hyd. No. 7

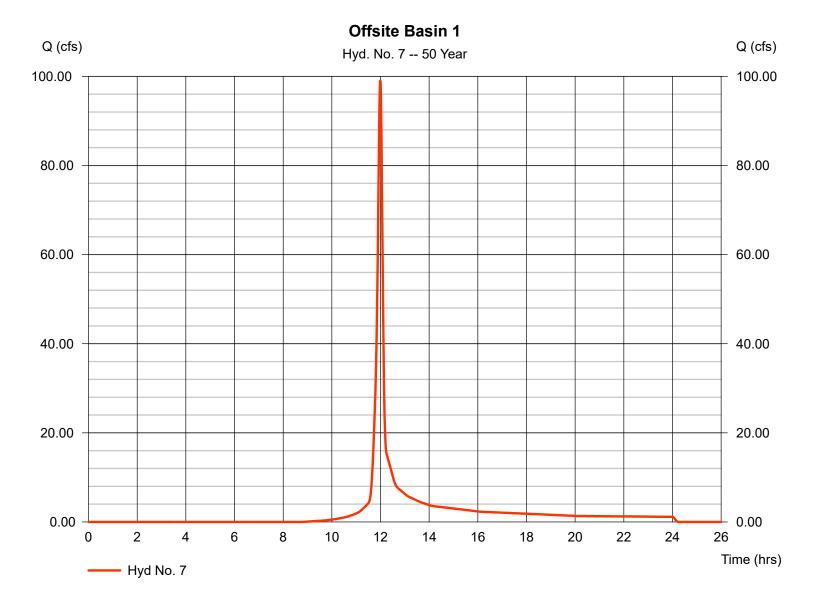
Offsite Basin 1

Hydrograph type = SCS Runoff Storm frequency = 50 yrsTime interval = 1 min Drainage area = 17.800 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 7.20 inStorm duration = 24 hrs

Peak discharge = 99.03 cfs
Time to peak = 11.98 hrs
Hyd. volume = 224,028 cuft
Curve number = 67
Hydraulic length = 0 ft
Time of conc. (Tc) = 8.80 min
Distribution = Type II

= 484

Shape factor



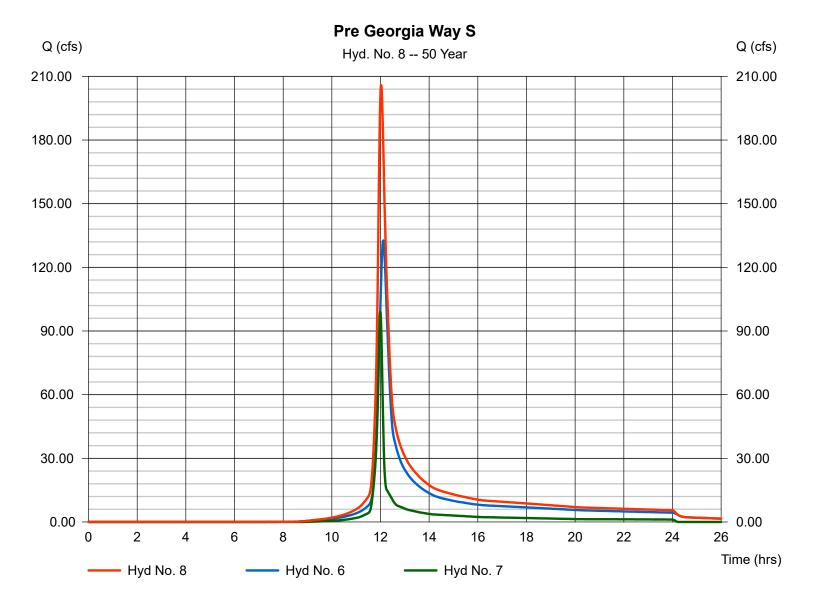
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 8

Pre Georgia Way S

Hydrograph type = Combine Storm frequency = 50 yrs Time interval = 1 min Inflow hyds. = 6, 7 Peak discharge = 205.95 cfs Time to peak = 12.02 hrs Hyd. volume = 873,214 cuft Contrib. drain. area = 17.800 ac



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Monday, Jul 10, 2017

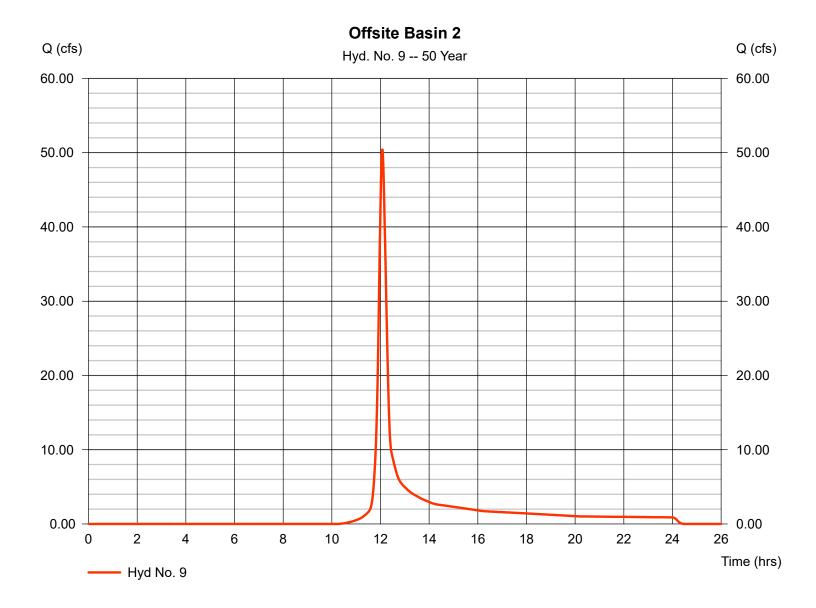
Hyd. No. 9

Offsite Basin 2

Hydrograph type = SCS Runoff Storm frequency = 50 yrsTime interval = 1 min Drainage area = 15.500 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 7.20 inStorm duration = 24 hrs

Peak discharge = 50.41 cfs
Time to peak = 12.08 hrs
Hyd. volume = 152,754 cuft
Curve number = 60
Hydraulic length = 0 ft

Time of conc. (Tc) = 17.30 min
Distribution = Type II
Shape factor = 484



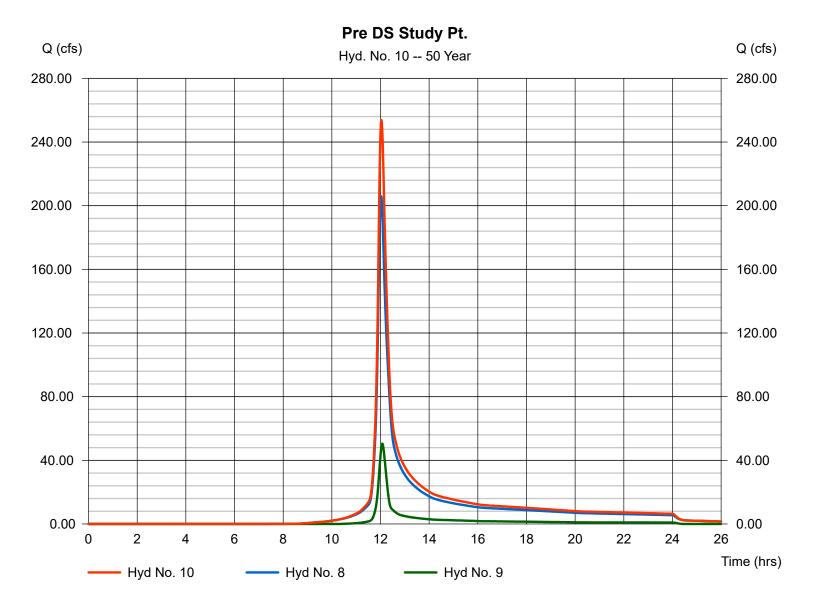
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Monday, Jul 10, 2017

Hyd. No. 10

Pre DS Study Pt.

Hydrograph type = Combine Storm frequency = 50 yrs Time interval = 1 min Inflow hyds. = 8, 9 Peak discharge = 253.81 cfs
Time to peak = 12.03 hrs
Hyd. volume = 1,025,967 cuft
Contrib. drain. area = 15.500 ac



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 12

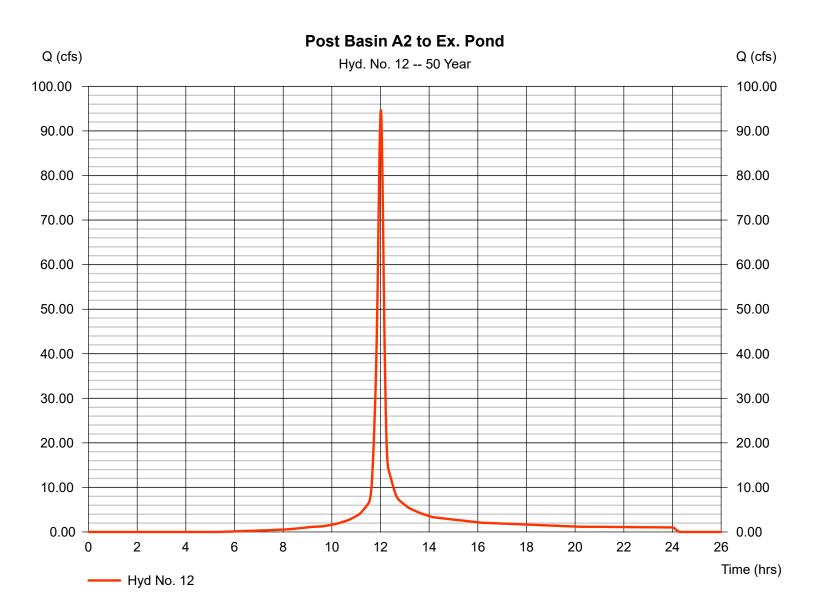
Post Basin A2 to Ex. Pond

Hydrograph type = SCS Runoff Storm frequency = 50 yrsTime interval = 1 min Drainage area = 13.730 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 7.20 inStorm duration = 24 hrs

Peak discharge = 94.64 cfs Time to peak = 12.02 hrs Hyd. volume = 244,883 cuft

Curve number = 81 Hydraulic length = 0 ft

Time of conc. (Tc) = 12.40 min
Distribution = Type II
Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

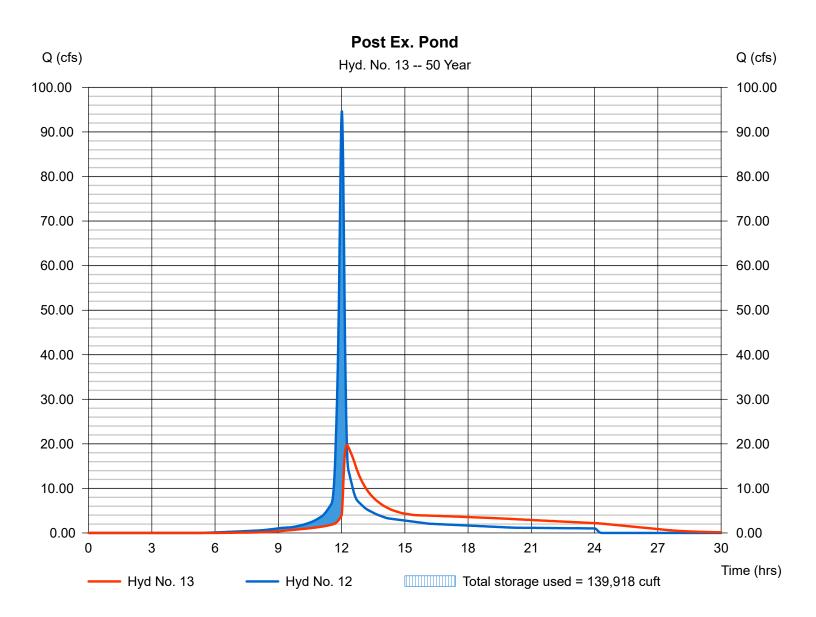
Monday, Jul 10, 2017

Hyd. No. 13

Post Ex. Pond

Hydrograph type = Reservoir Peak discharge = 19.74 cfsStorm frequency Time to peak = 50 yrs $= 12.25 \, hrs$ Time interval = 1 min Hyd. volume = 244,838 cuft Inflow hyd. No. = 12 - Post Basin A2 to Ex. Pond Max. Elevation $= 990.35 \, ft$ Reservoir name = Ex. Pond Max. Storage = 139,918 cuft

Storage Indication method used. Wet pond routing start elevation = 982.00 ft.



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 14

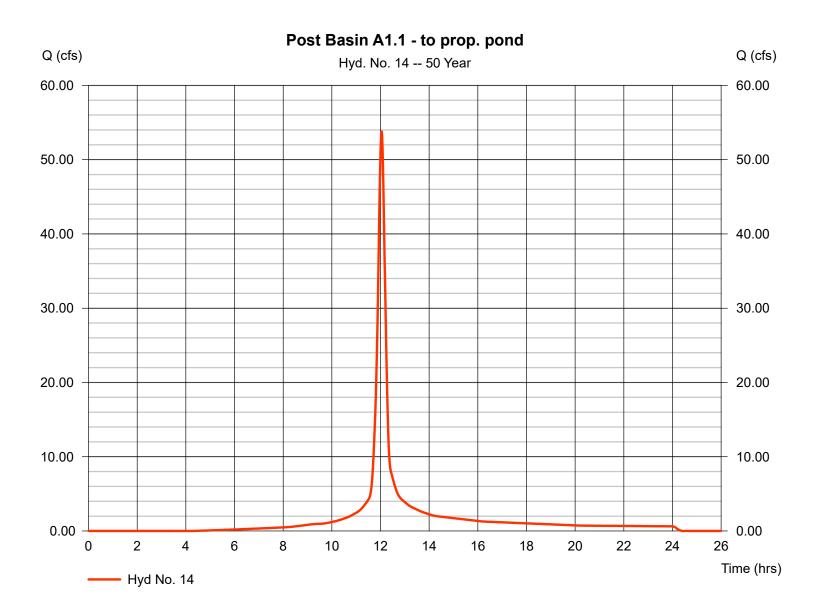
Post Basin A1.1 - to prop. pond

Hydrograph type = SCS Runoff Storm frequency = 50 yrsTime interval = 1 min Drainage area = 7.930 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 7.20 inStorm duration = 24 hrs

Peak discharge = 53.79 cfs
Time to peak = 12.05 hrs
Hyd. volume = 158,668 cuft
Curve number = 85

Hydraulic length = 0 ft
Time of conc. (Tc) = 16.30 min

Distribution = Type II Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

= 2.388 cfs

 $= 11.97 \, hrs$

= 4,796 cuft

 $= 5.00 \, \text{min}$

= Type II

= 61

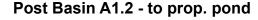
= 0 ft

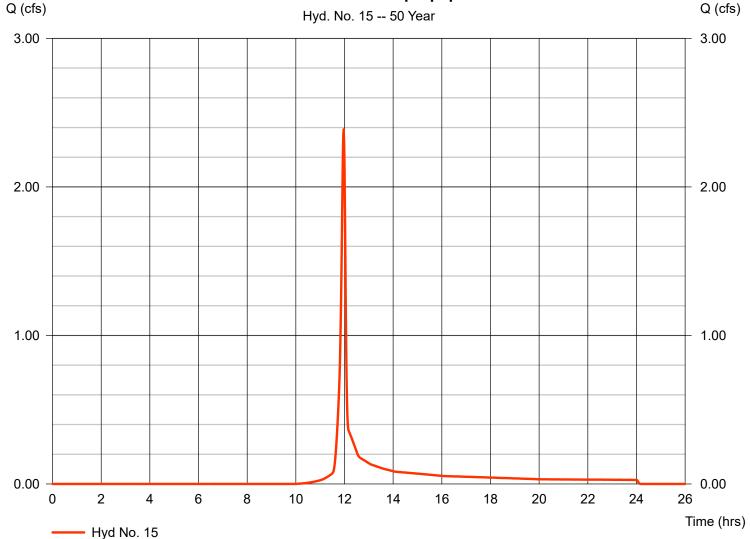
= 484

Hyd. No. 15

Post Basin A1.2 - to prop. pond

Hydrograph type = SCS Runoff Peak discharge Storm frequency Time to peak = 50 yrsTime interval = 1 min Hyd. volume Drainage area = 0.450 acCurve number Basin Slope = 0.0 % Hydraulic length Tc method = USER Time of conc. (Tc) Total precip. = 7.20 inDistribution Storm duration = 24 hrs Shape factor





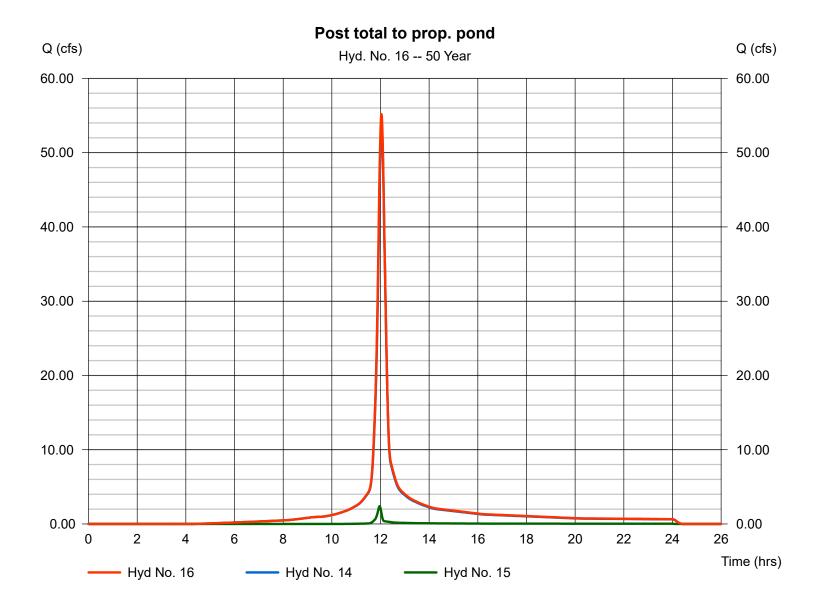
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Monday, Jul 10, 2017

Hyd. No. 16

Post total to prop. pond

Hydrograph type = Combine Storm frequency = 50 yrs Time interval = 1 min Inflow hyds. = 14, 15 Peak discharge = 55.19 cfs
Time to peak = 12.03 hrs
Hyd. volume = 163,465 cuft
Contrib. drain. area = 8.380 ac



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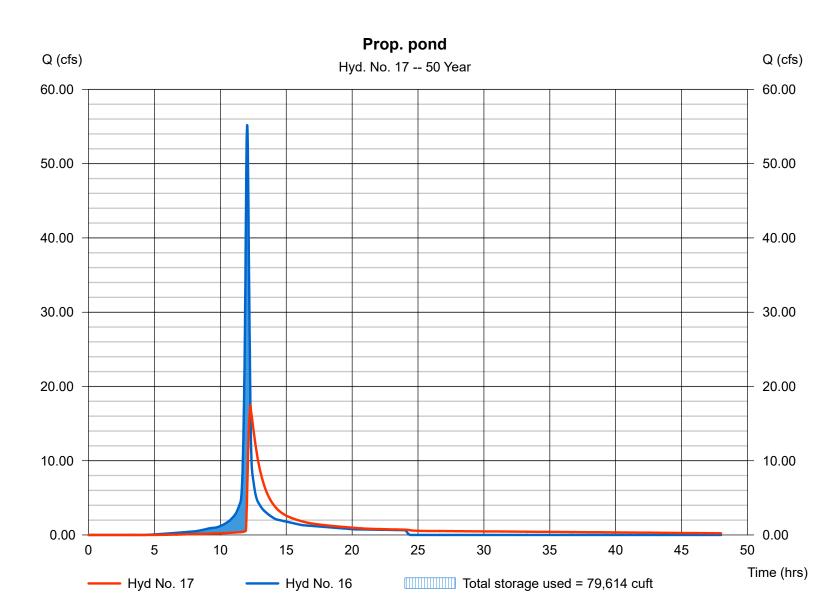
Monday, Jul 10, 2017

Hyd. No. 17

Prop. pond

Hydrograph type = Reservoir Peak discharge = 17.47 cfsStorm frequency Time to peak = 50 yrs $= 12.28 \, hrs$ Time interval = 1 min Hyd. volume = 156,740 cuftInflow hyd. No. = 16 - Post total to prop. pond Max. Elevation = 986.04 ftReservoir name = Stormwater Pond Max. Storage = 79,614 cuft

Storage Indication method used.



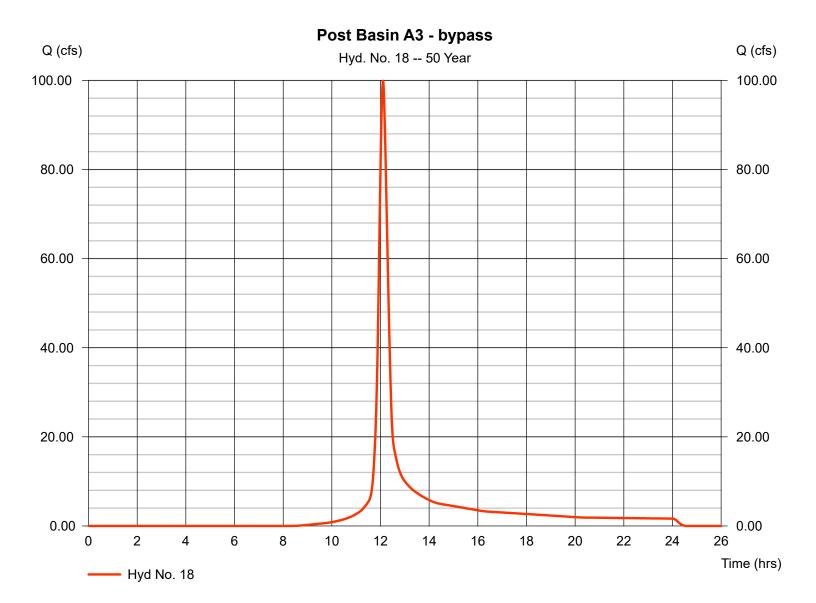
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Monday, Jul 10, 2017

Hyd. No. 18

Post Basin A3 - bypass

Hydrograph type = SCS Runoff Peak discharge = 99.91 cfsStorm frequency Time to peak = 50 yrs= 12.10 hrsTime interval = 1 min Hyd. volume = 328,825 cuft Drainage area = 24.390 acCurve number = 69 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 21.80 \, \text{min}$ Total precip. = 7.20 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



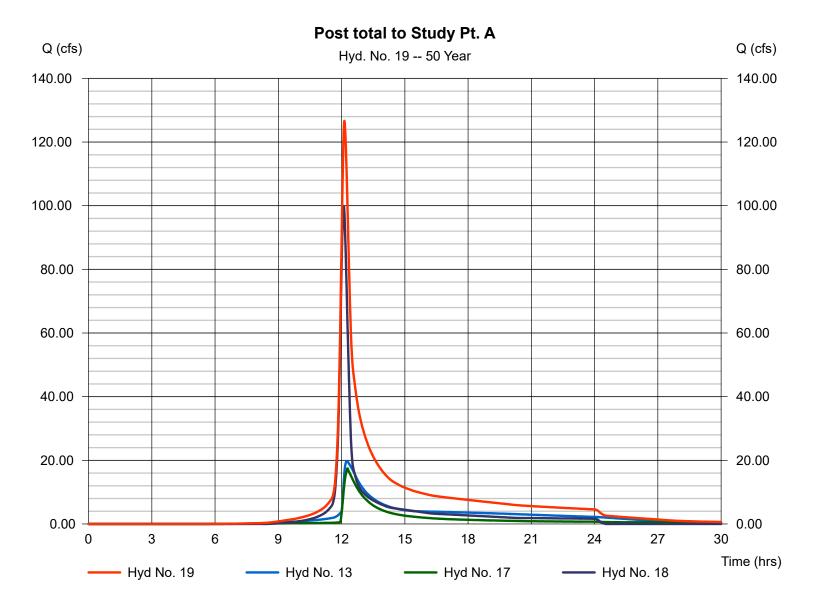
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Monday, Jul 10, 2017

Hyd. No. 19

Post total to Study Pt. A

Hydrograph type = Combine Storm frequency = 50 yrs Time interval = 1 min Inflow hyds. = 13, 17, 18 Peak discharge = 126.62 cfs Time to peak = 12.13 hrs Hyd. volume = 730,403 cuft Contrib. drain. area = 24.390 ac



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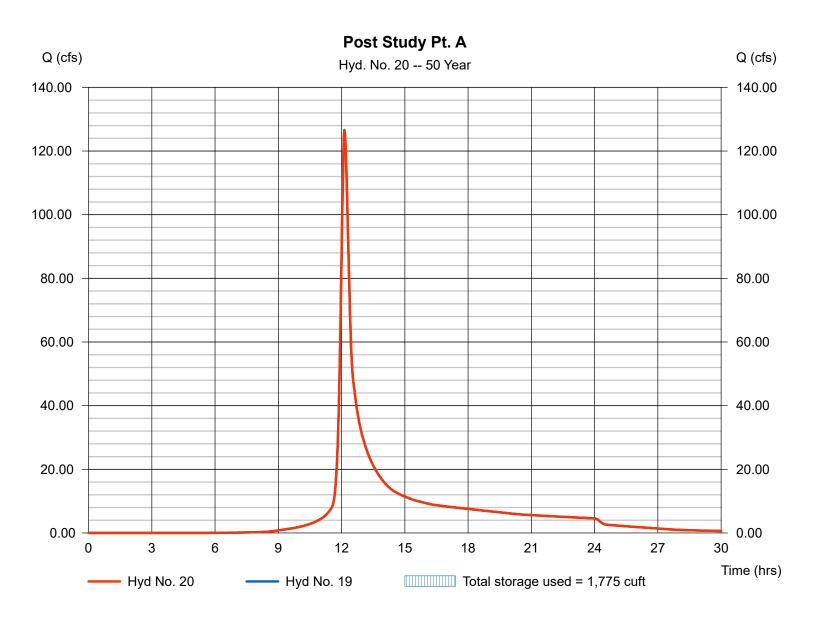
Monday, Jul 10, 2017

Hyd. No. 20

Post Study Pt. A

Hydrograph type = Reservoir Peak discharge = 126.38 cfsStorm frequency Time to peak = 50 yrs $= 12.15 \, hrs$ Time interval = 1 min Hyd. volume = 730,397 cuftInflow hyd. No. = 19 - Post total to Study Pt. A Max. Elevation $= 973.75 \, \text{ft}$ Reservoir name = Ex. DS Culvert at Barclay Max. Storage = 1,775 cuft

Storage Indication method used.



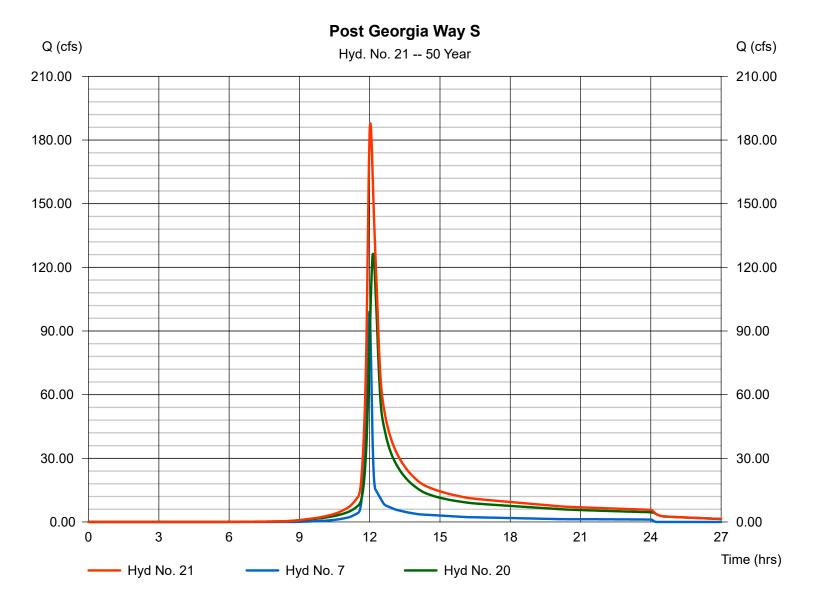
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 21

Post Georgia Way S

Hydrograph type = Combine Storm frequency = 50 yrs Time interval = 1 min Inflow hyds. = 7, 20 Peak discharge = 187.86 cfs Time to peak = 12.03 hrs Hyd. volume = 954,425 cuft Contrib. drain. area = 17.800 ac



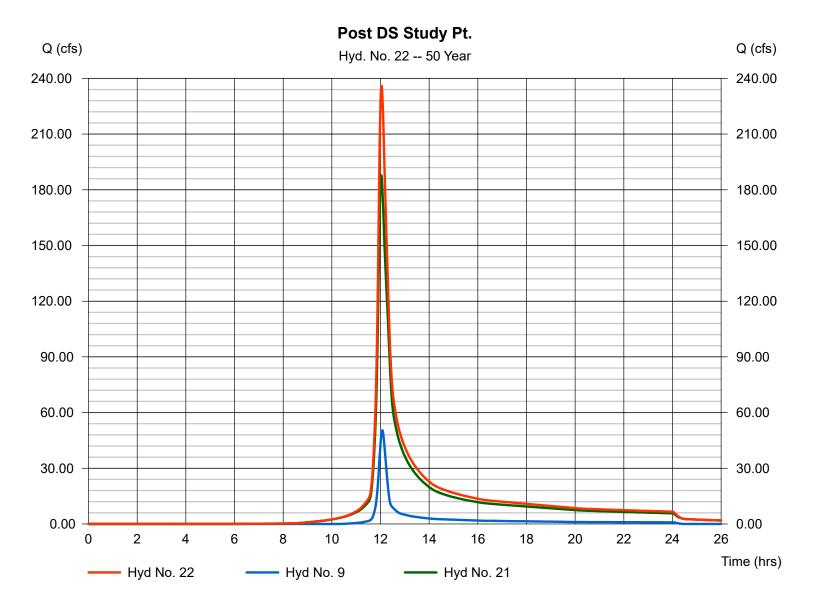
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 22

Post DS Study Pt.

Hydrograph type = Combine Storm frequency = 50 yrs Time interval = 1 min Inflow hyds. = 9, 21 Peak discharge = 235.96 cfs
Time to peak = 12.03 hrs
Hyd. volume = 1,107,179 cuft
Contrib. drain. area = 15.500 ac



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

| Hyd. No. | Hydrograph type (origin) | Peak flow (cfs) | Time interval (min) | Time to peak (min) | Hyd. volume (cuft) | Inflow hyd(s) | Maximum elevation (ft) | Total strge used (cuft) | Hydrograph description | | | |
|-------------|--------------------------------|-----------------------|---------------------------|--------------------|--------------------------|------------------|------------------------------|-------------------------------|-------------------------------------|--|--|--|
| 1 | SCS Runoff | 118.09 | 1 | 721 | 303,993 | | | | Pre Basin A2- to Ex. Detention Pond | | | |
| 2 | Reservoir | 32.13 | 1 | 733 | 303,939 | 1 | 991.48 | 160,769 | Pre Ex. Pond | | | |
| 3 | SCS Runoff | 31.02 | 1 | 721 | 79,131 | | | | Pre Basin A1- site | | | |
| 4 | SCS Runoff | 112.23 | 1 | 726 | 368,765 | | | | Pre Basin A3 - bypass | | | |
| 5 | Combine | 161.43 | 1 | 726 | 751,834 | 2, 3, 4 | | | Pre total to Study Pt. A | | | |
| 6 | Reservoir | 158.40 | 1 | 728 | 751,834 | 5 | 975.30 | 4,936 | Pre Study Pt. A | | | |
| 7 | SCS Runoff | 115.75 | 1 | 719 | 262,008 | | | | Offsite Basin 1 | | | |
| 8 | Combine | 243.65 | 1 | 721 | 1,013,843 | 6, 7 | | | Pre Georgia Way S | | | |
| 9 | SCS Runoff | 60.65 | 1 | 724 | 182,088 | | | | Offsite Basin 2 | | | |
| 10 | Combine | 299.36 | 1 | 721 | 1,195,931 | 8, 9 | | | Pre DS Study Pt. | | | |
| 12 | SCS Runoff | 106.80 | 1 | 721 | 278,021 | | | | Post Basin A2 to Ex. Pond | | | |
| 13 | Reservoir | 26.91 | 1 | 734 | 277,972 | 12 | 991.03 | 152,458 | Post Ex. Pond | | | |
| 14 | SCS Runoff | 60.25 | 1 | 723 | 178,840 | | | | Post Basin A1.1 - to prop. pond | | | |
| 15 | SCS Runoff | 2.838 | 1 | 718 | 5,700 | | | | Post Basin A1.2 - to prop. pond | | | |
| 16 | Combine | 61.94 | 1 | 722 | 184,540 | 14, 15 | | | Post total to prop. pond | | | |
| 17 | Reservoir | 32.10 | 1 | 733 | 177,673 | 16 | 986.48 | 84,058 | Prop. pond | | | |
| 18 | SCS Runoff | 116.48 | 1 | 726 | 382,731 | | | | Post Basin A3 - bypass | | | |
| 19 | Combine | 160.18 | 1 | 730 | 838,376 | 13, 17, 18 | | | Post total to Study Pt. A | | | |
| 20 | Reservoir | 157.42 | 1 | 732 | 838,371 | 19 | 975.26 | 4,814 | Post Study Pt. A | | | |
| 21 | Combine | 224.11 | 1 | 722 | 1,100,378 | 7, 20 | | | Post Georgia Way S | | | |
| 22 | Combine | 282.80 | 1 | 723 | 1,282,466 | 9, 21 | | | Post DS Study Pt. | | | |
| 24 | Reservoir | 36.50 | 1 | 731 | 145,767 | 16 | 986.59 | 85,189 | Emergency Overflow | | | |
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07-11-17.gpw Return Period: 100 Year Monday, Jul 10, 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 1

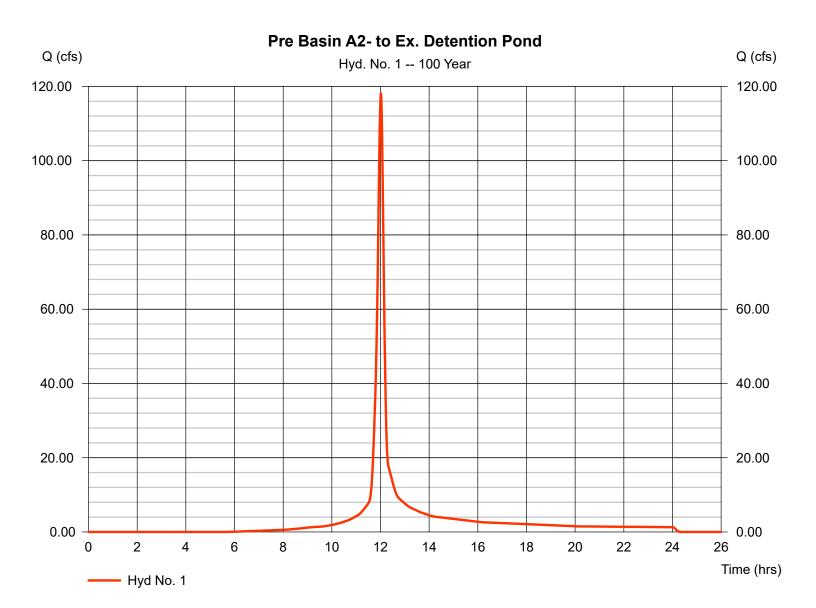
Pre Basin A2- to Ex. Detention Pond

= SCS Runoff Hydrograph type Storm frequency = 100 yrsTime interval = 1 min Drainage area = 16.000 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 7.92 inStorm duration = 24 hrs

Peak discharge = 118.09 cfs
Time to peak = 12.02 hrs
Hyd. volume = 303,993 cuft
Curve number = 78

Curve number = 78Hydraulic length = 0 ft

Time of conc. (Tc) = 12.40 min
Distribution = Type II
Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

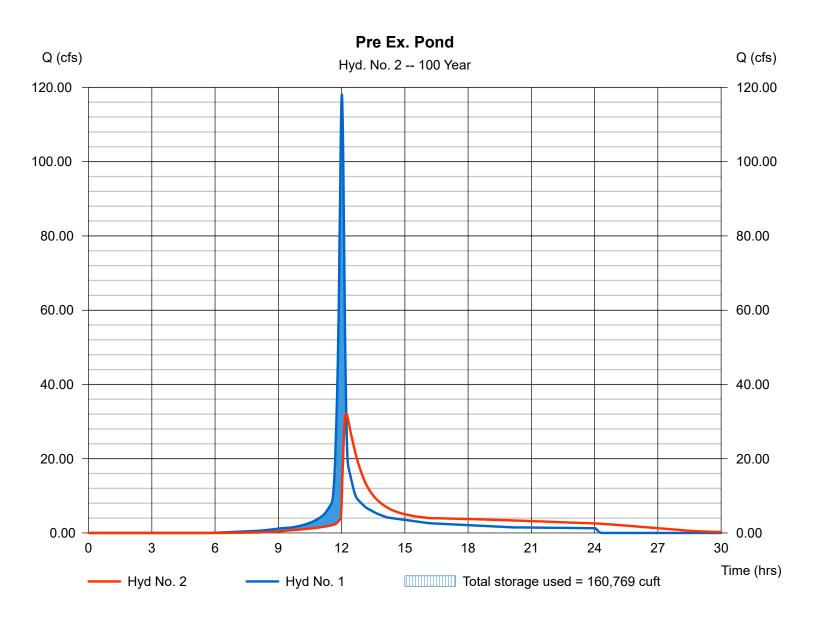
Monday, Jul 10, 2017

Hyd. No. 2

Pre Ex. Pond

Hydrograph type = Reservoir Peak discharge = 32.13 cfsStorm frequency Time to peak = 100 yrs $= 12.22 \, hrs$ Time interval = 1 min Hyd. volume = 303,939 cuftInflow hyd. No. = 1 - Pre Basin A2- to Ex. Detention Pond Max. Elevation = 991.48 ftReservoir name = Ex. Pond Max. Storage = 160,769 cuft

Storage Indication method used. Wet pond routing start elevation = 982.00 ft.



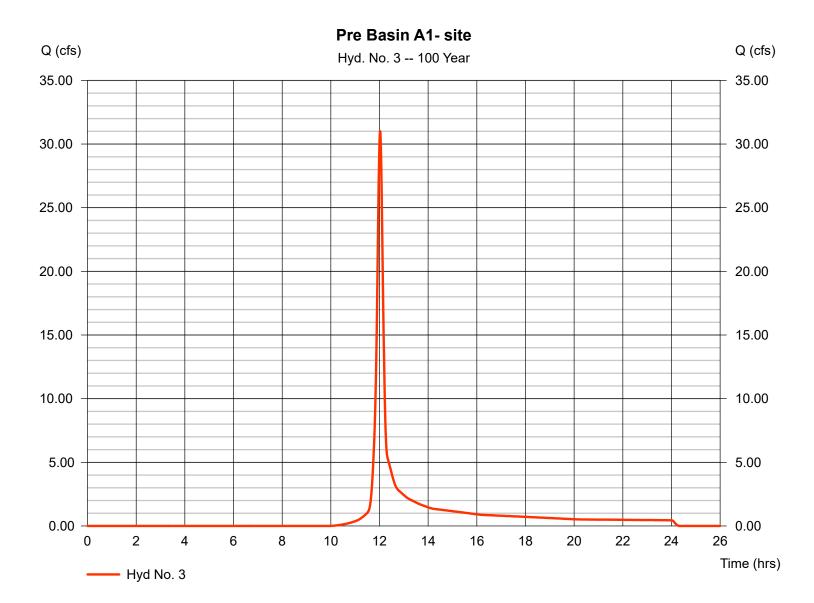
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 3

Pre Basin A1- site

Hydrograph type = SCS Runoff Peak discharge = 31.02 cfsStorm frequency Time to peak = 100 yrs $= 12.02 \, hrs$ Time interval = 1 min Hyd. volume = 79,131 cuft Drainage area = 7.000 acCurve number = 59 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 12.80 \, \text{min}$ Distribution Total precip. = 7.92 in= Type II Storm duration = 484 = 24 hrs Shape factor



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 4

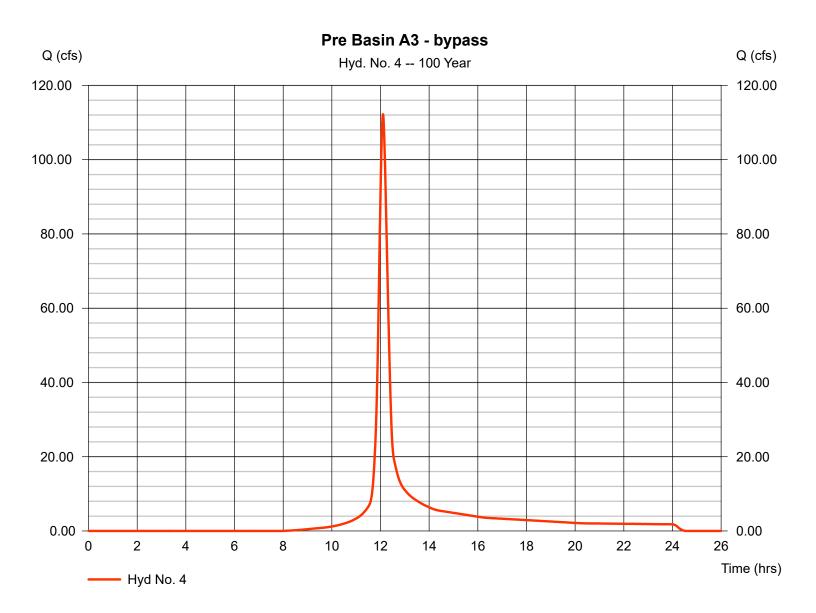
Pre Basin A3 - bypass

Hydrograph type = SCS Runoff Storm frequency = 100 yrsTime interval = 1 min Drainage area = 23.500 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 7.92 inStorm duration = 24 hrs

Peak discharge = 112.23 cfs
Time to peak = 12.10 hrs
Hyd. volume = 368,765 cuft

Curve number = 69 Hydraulic length = 0 ft

Time of conc. (Tc) = 21.80 min
Distribution = Type II
Shape factor = 484



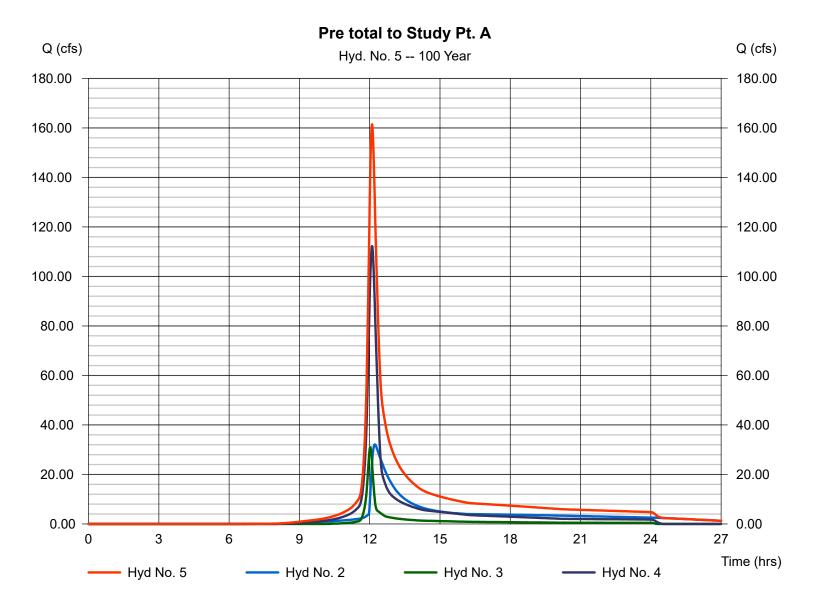
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Monday, Jul 10, 2017

Hyd. No. 5

Pre total to Study Pt. A

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 1 min Inflow hyds. = 2, 3, 4 Peak discharge = 161.43 cfs Time to peak = 12.10 hrs Hyd. volume = 751,834 cuft Contrib. drain. area = 30.500 ac



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

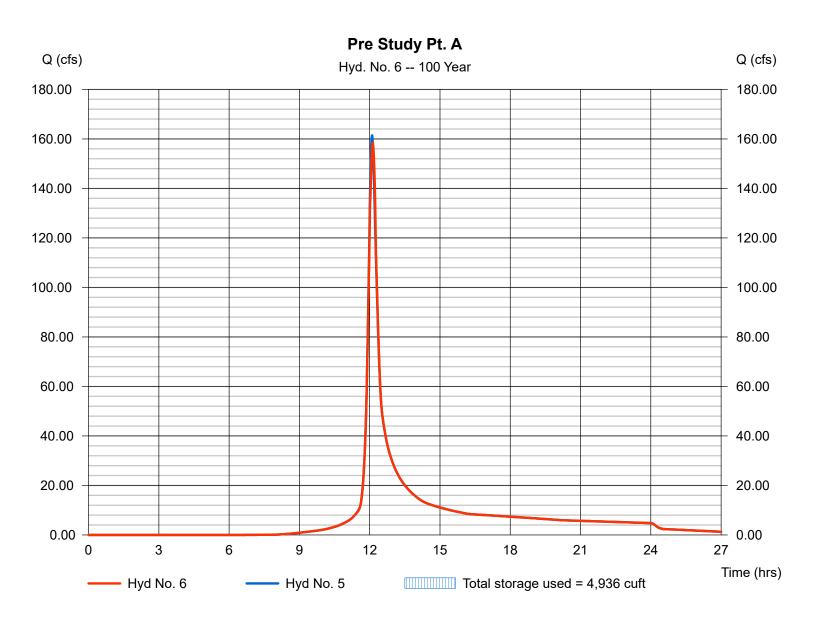
Monday, Jul 10, 2017

Hyd. No. 6

Pre Study Pt. A

Hydrograph type = Reservoir Peak discharge = 158.40 cfsStorm frequency Time to peak = 100 yrs $= 12.13 \, hrs$ Time interval = 1 min Hyd. volume = 751,834 cuft Inflow hyd. No. = 5 - Pre total to Study Pt. A Max. Elevation = 975.30 ftReservoir name = Ex. DS Culvert at Barclay Max. Storage = 4,936 cuft

Storage Indication method used.



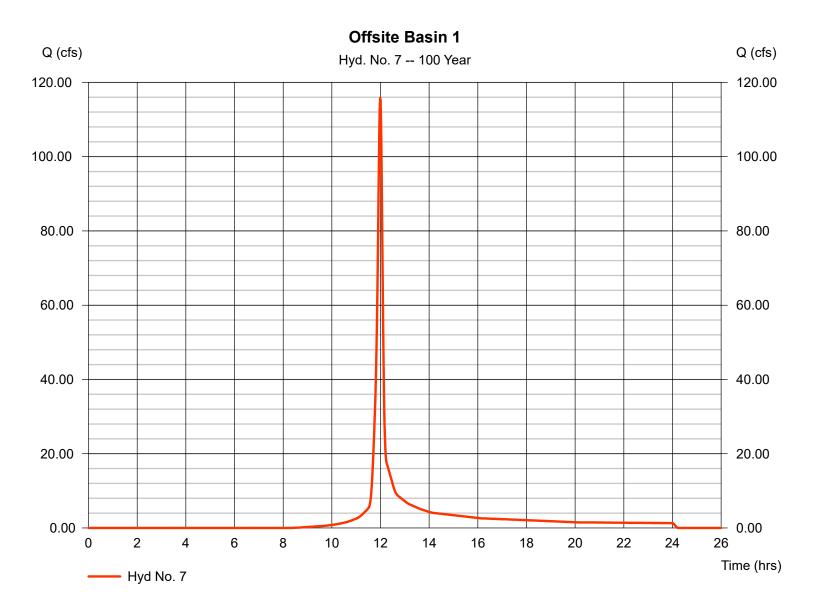
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 7

Offsite Basin 1

Hydrograph type = SCS Runoff Peak discharge = 115.75 cfsStorm frequency Time to peak = 100 yrs= 11.98 hrsTime interval = 1 min Hyd. volume = 262,008 cuft Drainage area = 17.800 acCurve number = 67 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.80 \, \text{min}$ Distribution Total precip. = 7.92 in= Type II Storm duration = 24 hrs Shape factor = 484



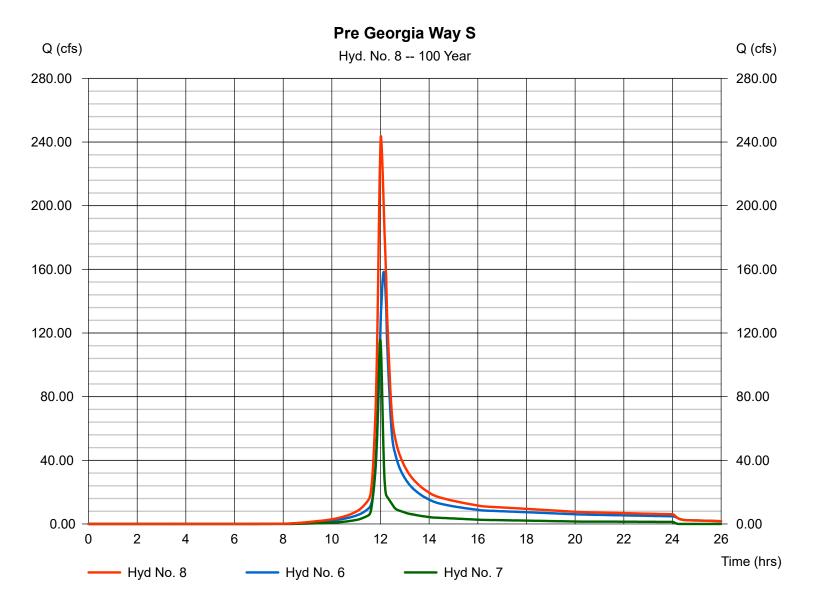
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 8

Pre Georgia Way S

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 1 min Inflow hyds. = 6, 7 Peak discharge = 243.65 cfs Time to peak = 12.02 hrs Hyd. volume = 1,013,843 cuft Contrib. drain. area = 17.800 ac



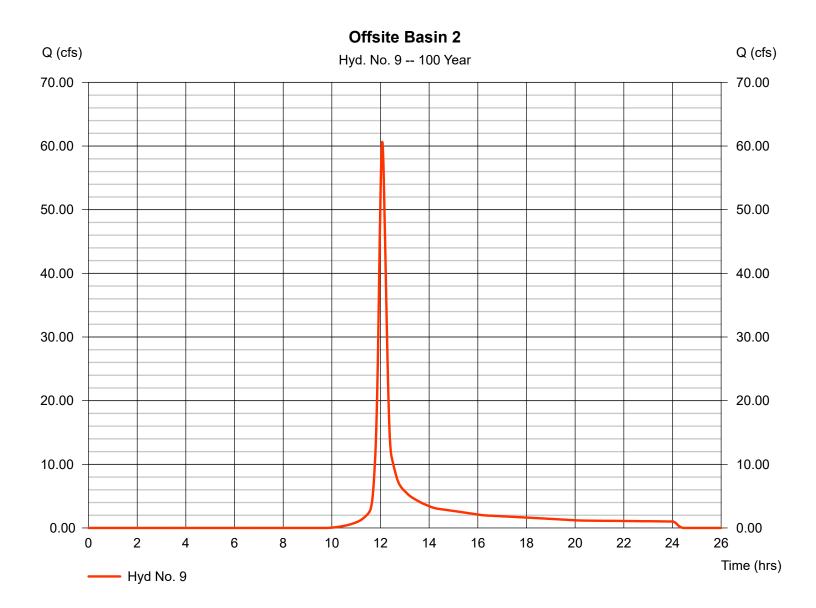
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 9

Offsite Basin 2

Hydrograph type = SCS Runoff Peak discharge = 60.65 cfsStorm frequency Time to peak = 100 yrs= 12.07 hrsTime interval = 1 min Hyd. volume = 182,088 cuft Drainage area = 15.500 acCurve number = 60 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 17.30 minDistribution Total precip. = 7.92 in= Type II Storm duration = 24 hrs Shape factor = 484



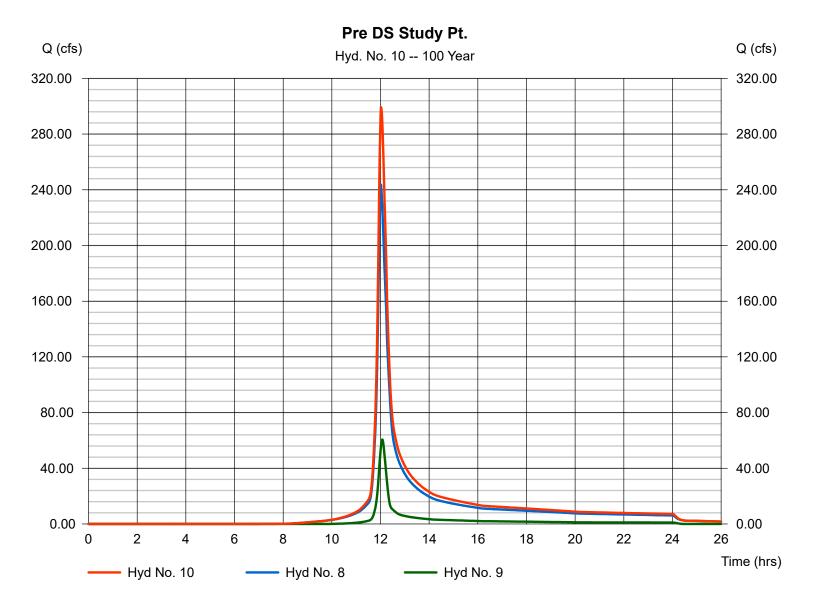
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Hyd. No. 10

Pre DS Study Pt.

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 1 min Inflow hyds. = 8, 9 Peak discharge = 299.36 cfs
Time to peak = 12.02 hrs
Hyd. volume = 1,195,931 cuft
Contrib. drain. area = 15.500 ac



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Monday, Jul 10, 2017

Hyd. No. 12

Post Basin A2 to Ex. Pond

= SCS Runoff Hydrograph type Storm frequency = 100 yrsTime interval = 1 min Drainage area = 13.730 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 7.92 inStorm duration = 24 hrs

Peak discharge = 106.80 cfs
Time to peak = 12.02 hrs
Hyd. volume = 278,021 cuft
Curve number = 81

Curve number = 81 Hydraulic length = 0 ft Time of conc. (Tc) = 12.40 min

Distribution = Type II
Shape factor = 484

Post Basin A2 to Ex. Pond Q (cfs) Q (cfs) Hyd. No. 12 -- 100 Year 120.00 120.00 100.00 100.00 80.00 80.00 60.00 60.00 40.00 40.00 20.00 20.00 0.00 0.00 2 4 6 8 10 12 14 16 18 22 24 26 20 Time (hrs) Hyd No. 12

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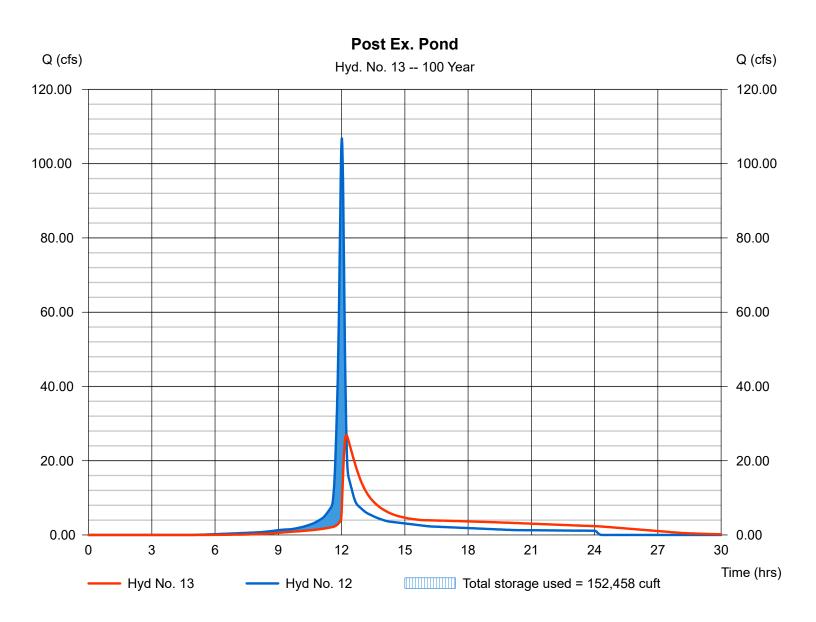
Monday, Jul 10, 2017

Hyd. No. 13

Post Ex. Pond

Hydrograph type = Reservoir Peak discharge = 26.91 cfsStorm frequency Time to peak = 100 yrs $= 12.23 \, hrs$ Time interval = 1 min Hyd. volume = 277,972 cuft Inflow hyd. No. = 12 - Post Basin A2 to Ex. Pond Max. Elevation = 991.03 ftReservoir name = Ex. Pond Max. Storage = 152,458 cuft

Storage Indication method used. Wet pond routing start elevation = 982.00 ft.



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Hyd. No. 14

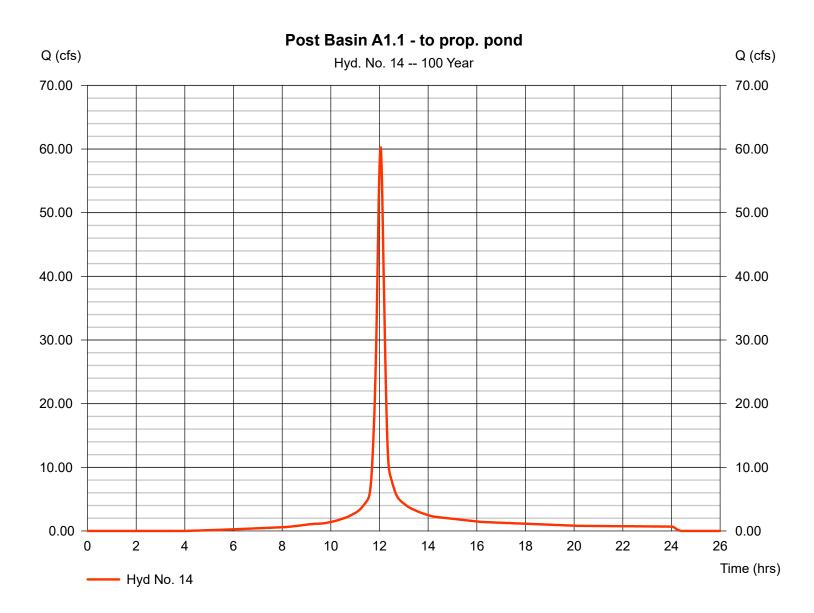
Post Basin A1.1 - to prop. pond

= SCS Runoff Hydrograph type Storm frequency = 100 yrsTime interval = 1 min Drainage area = 7.930 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 7.92 inStorm duration = 24 hrs

Peak discharge = 60.25 cfs
Time to peak = 12.05 hrs
Hyd. volume = 178,840 cuft

Curve number = 85Hydraulic length = 0 ft

Time of conc. (Tc) = 16.30 min
Distribution = Type II
Shape factor = 484



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= 2.838 cfs

 $= 11.97 \, hrs$

= 5,700 cuft

 $= 5.00 \, \text{min}$

= Type II

= 61

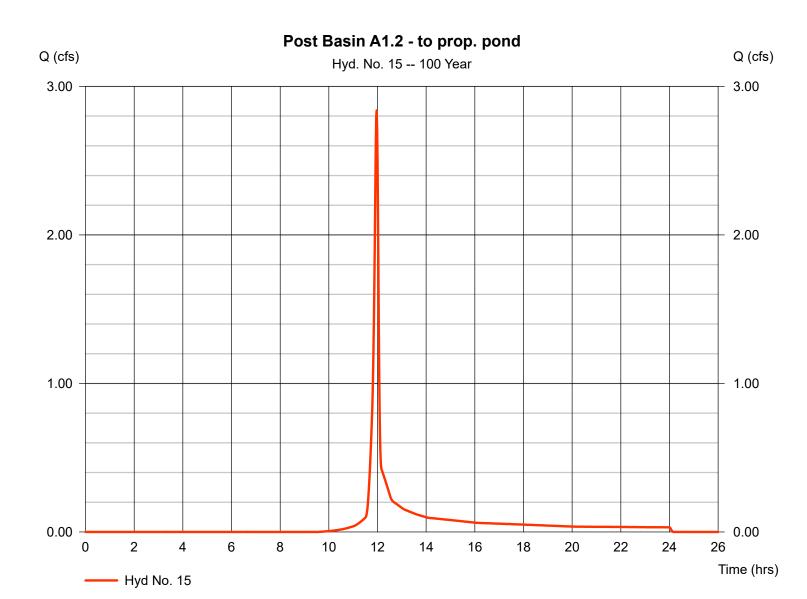
= 0 ft

= 484

Hyd. No. 15

Post Basin A1.2 - to prop. pond

Hydrograph type = SCS Runoff Peak discharge Storm frequency Time to peak = 100 yrsTime interval = 1 min Hyd. volume Drainage area = 0.450 acCurve number Basin Slope = 0.0 % Hydraulic length Tc method = USER Time of conc. (Tc) Total precip. = 7.92 inDistribution Storm duration = 24 hrs Shape factor



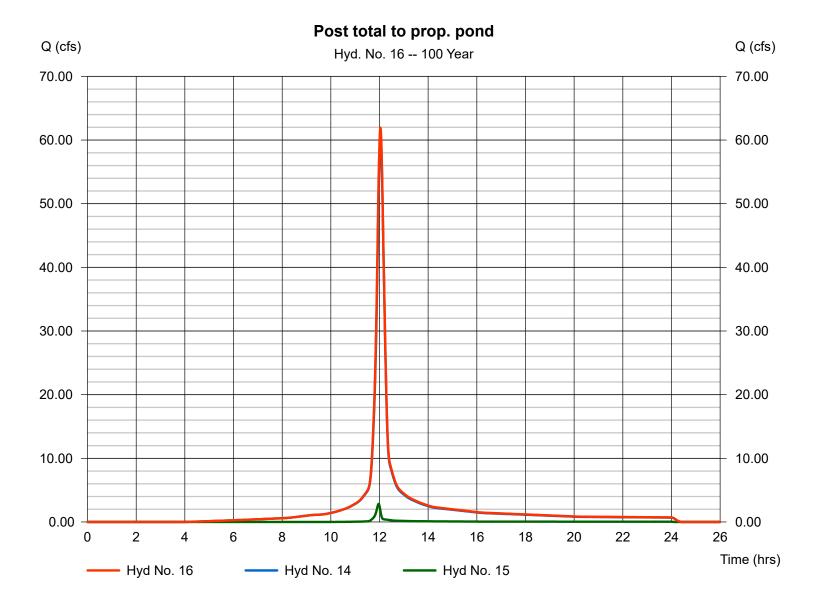
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Hyd. No. 16

Post total to prop. pond

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 1 min Inflow hyds. = 14, 15 Peak discharge = 61.94 cfs
Time to peak = 12.03 hrs
Hyd. volume = 184,540 cuft
Contrib. drain. area = 8.380 ac



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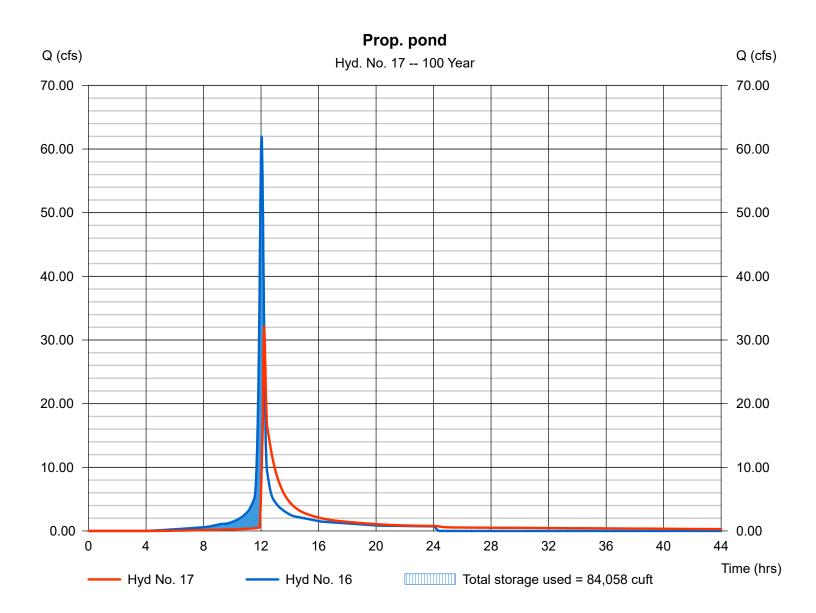
Monday, Jul 10, 2017

Hyd. No. 17

Prop. pond

Hydrograph type = Reservoir Peak discharge = 32.10 cfsStorm frequency Time to peak = 100 yrs $= 12.22 \, hrs$ Time interval = 1 min Hyd. volume = 177,673 cuft Inflow hyd. No. = 16 - Post total to prop. pond Max. Elevation = 986.48 ftReservoir name = Stormwater Pond Max. Storage = 84,058 cuft

Storage Indication method used.



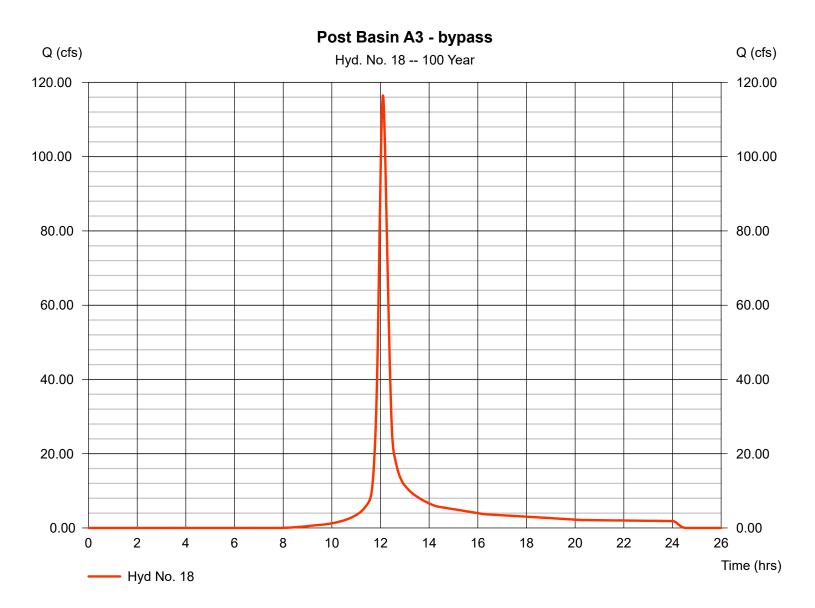
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Hyd. No. 18

Post Basin A3 - bypass

Hydrograph type = SCS Runoff Peak discharge = 116.48 cfsStorm frequency Time to peak = 100 yrs= 12.10 hrsTime interval = 1 min Hyd. volume = 382,731 cuftDrainage area = 24.390 acCurve number = 69 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 21.80 \, \text{min}$ Distribution Total precip. = 7.92 in= Type II Storm duration = 24 hrs Shape factor = 484



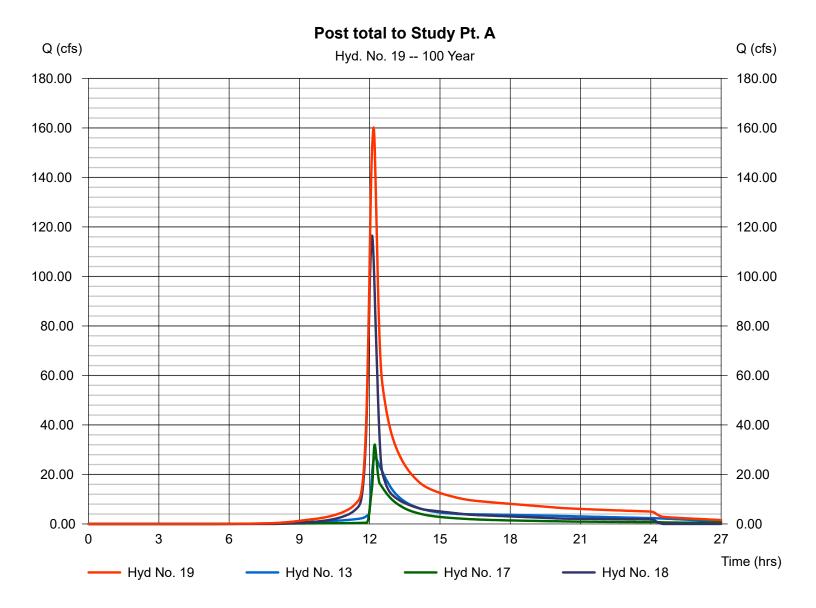
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Monday, Jul 10, 2017

Hyd. No. 19

Post total to Study Pt. A

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 1 min Inflow hyds. = 13, 17, 18 Peak discharge = 160.18 cfs Time to peak = 12.17 hrs Hyd. volume = 838,376 cuft Contrib. drain. area = 24.390 ac



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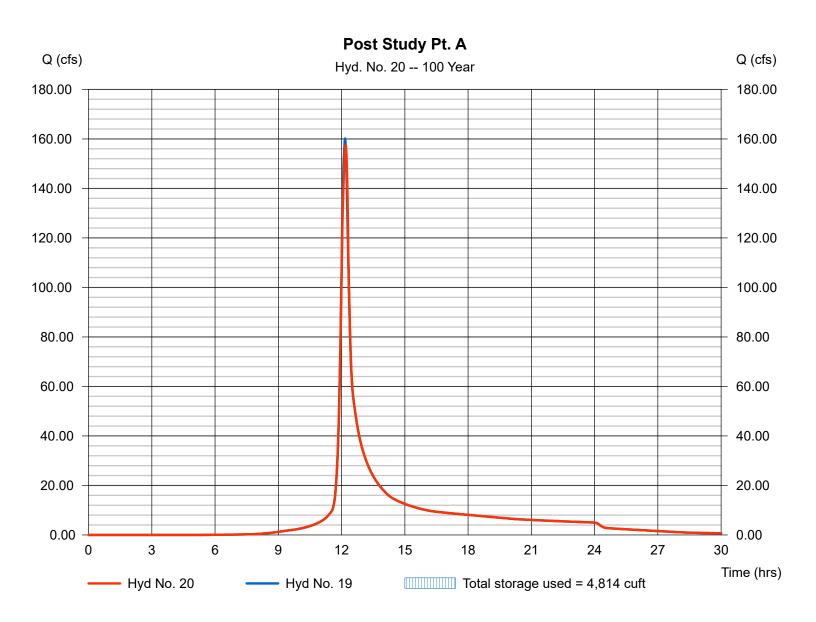
Monday, Jul 10, 2017

Hyd. No. 20

Post Study Pt. A

Hydrograph type = Reservoir Peak discharge = 157.42 cfsStorm frequency Time to peak = 100 yrs= 12.20 hrsTime interval = 1 min Hyd. volume = 838,371 cuft Inflow hyd. No. = 19 - Post total to Study Pt. A Max. Elevation = 975.26 ftReservoir name = Ex. DS Culvert at Barclay Max. Storage = 4,814 cuft

Storage Indication method used.



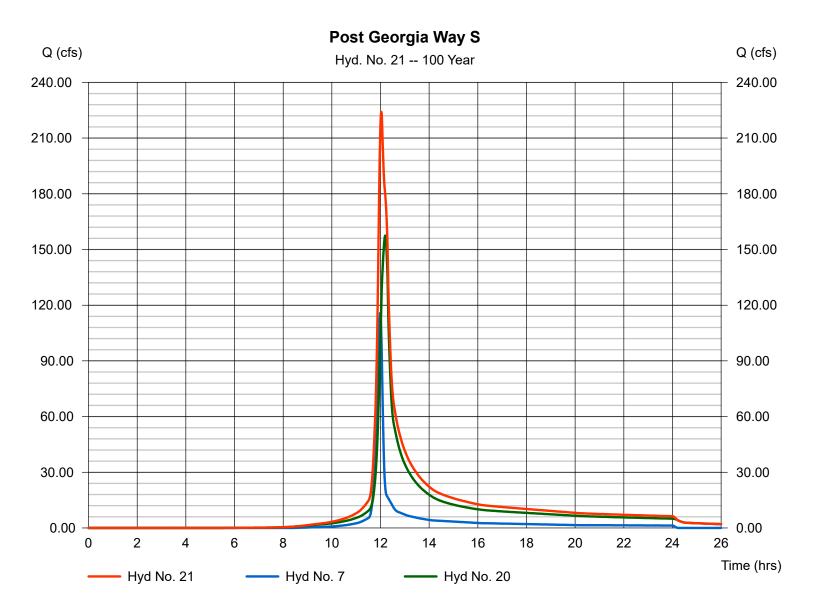
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Monday, Jul 10, 2017

Hyd. No. 21

Post Georgia Way S

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 1 min Inflow hyds. = 7, 20 Peak discharge = 224.11 cfs
Time to peak = 12.03 hrs
Hyd. volume = 1,100,378 cuft
Contrib. drain. area = 17.800 ac



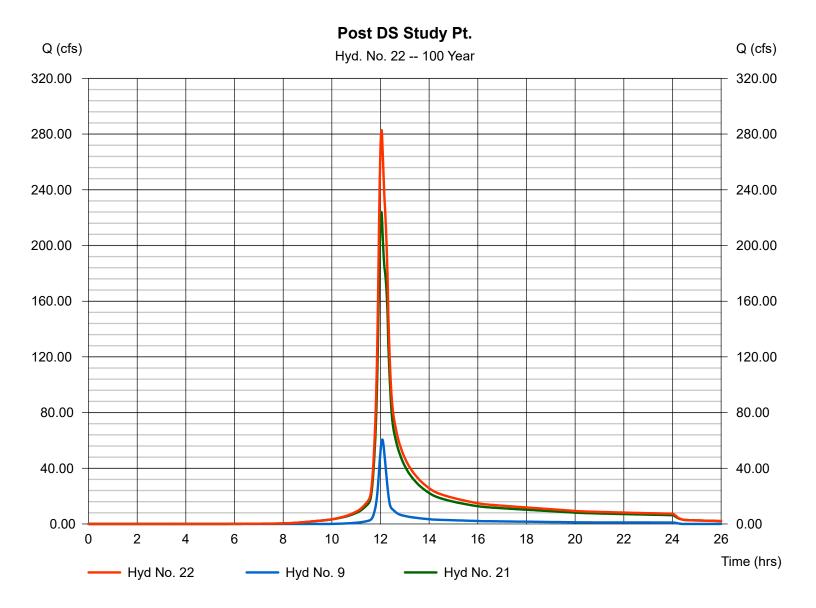
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Monday, Jul 10, 2017

Hyd. No. 22

Post DS Study Pt.

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 1 min Inflow hyds. = 9, 21 Peak discharge = 282.80 cfs Time to peak = 12.05 hrs Hyd. volume = 1,282,466 cuft Contrib. drain. area = 15.500 ac



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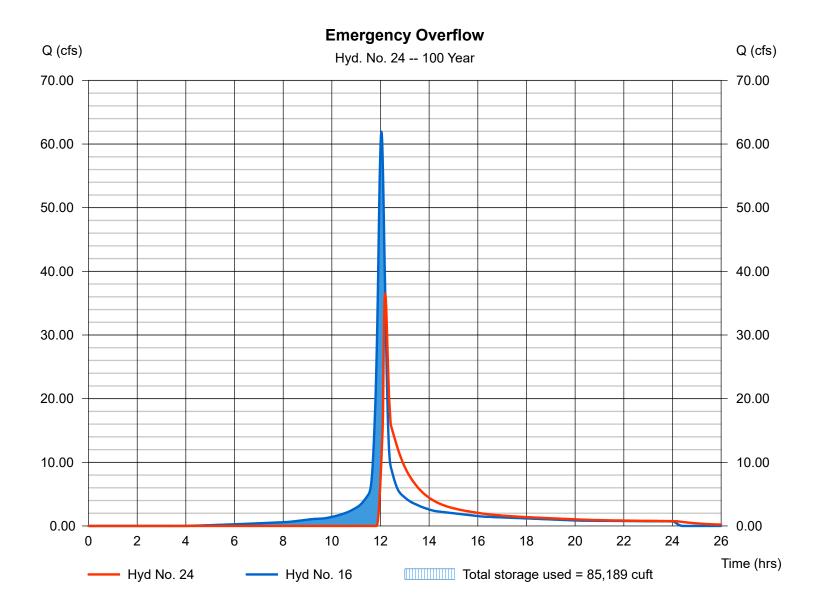
Monday, Jul 10, 2017

Hyd. No. 24

Emergency Overflow

Hydrograph type = Reservoir Peak discharge = 36.50 cfsStorm frequency Time to peak = 100 yrs $= 12.18 \, hrs$ Time interval = 1 min Hyd. volume = 145,767 cuft Inflow hyd. No. = 16 - Post total to prop. pond Max. Elevation = 986.59 ftReservoir name = Prop. Pond EO Max. Storage = 85,189 cuft

Storage Indication method used.



Pond Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Jul 10, 2017

Pond No. 16 - Prop. Pond EO

Pond Data

Contours - User-defined contour areas. Conic method used for volume calculation. Begining Elevation = 976.00 ft

Stage / Storage Table

| Stage (ft) | Elevation (ft) | Contour area (sqft) | Incr. Storage (cuft) | Total storage (cuft) | | |
|------------|----------------|---------------------|----------------------|----------------------|--|--|
| 0.00 | 976.00 | 6,240 | 0 | 0 | | |
| 2.00 | 978.00 | 6,887 | 13,120 | 13,120 | | |
| 4.00 | 980.00 | 7,555 | 14,435 | 27,556 | | |
| 6.00 | 982.00 | 8,252 | 15,800 | 43,356 | | |
| 8.00 | 984.00 | 8,968 | 17,213 | 60,569 | | |
| 10.00 | 986.00 | 9,708 | 18,669 | 79,239 | | |
| 12.00 | 988.00 | 10,475 | 20,176 | 99,415 | | |

Culvert / Orifice Structures

Weir Structures

| | [A] | [B] | [C] | [PrfRsr] | | [A] | [B] | [C] | [D] |
|-----------------|----------|------|------|----------|----------------|-------------|-----------|------|------|
| Rise (in) | = 30.00 | 0.00 | 0.00 | 0.00 | Crest Len (ft) | = 11.50 | 0.50 | 0.00 | 0.00 |
| Span (in) | = 30.00 | 0.00 | 0.00 | 0.00 | Crest El. (ft) | = 986.00 | 981.50 | 0.00 | 0.00 |
| No. Barrels | = 1 | 0 | 0 | 0 | Weir Coeff. | = 3.33 | 3.33 | 3.33 | 3.33 |
| Invert El. (ft) | = 976.00 | 0.00 | 0.00 | 0.00 | Weir Type | = Riser | Rect | | |
| Length (ft) | = 0.00 | 0.00 | 0.00 | 0.00 | Multi-Stage | = Yes | Yes | No | No |
| Slope (%) | = 0.00 | 0.00 | 0.00 | n/a | | | | | |
| N-Value | = .013 | .013 | .013 | n/a | | | | | |
| Orifice Coeff. | = 0.60 | 0.60 | 0.60 | 0.60 | Exfil.(in/hr) | = 0.000 (by | Wet area) | | |
| Multi-Stage | = n/a | No | No | No | TW Elev. (ft) | = 0.00 | | | |

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Stage / Storage / Discharge Table

| Stage ft | Storage cuft | Elevation ft | CIv A cfs | CIv B cfs | CIv C cfs | PrfRsr cfs | Wr A cfs | Wr B cfs | Wr C cfs | Wr D cfs | Exfil cfs | User cfs | Total cfs |
|-------------|--------------|-----------------|--------------|--------------|--------------|---------------|-------------|-------------|-------------|-------------|--------------|-------------|--------------|
| 0.00 | 0 | 976.00 | 0.00 | | | | 0.00 | 0.00 | | | | | 0.000 |
| 2.00 | 13,120 | 978.00 | 0.00 | | | | 0.00 | 0.00 | | | | | 0.000 |
| 4.00 | 27,556 | 980.00 | 0.00 | | | | 0.00 | 0.00 | | | | | 0.000 |
| 6.00 | 43,356 | 982.00 | 0.61 ic | | | | 0.00 | 0.59 | | | | | 0.589 |
| 8.00 | 60,569 | 984.00 | 6.59 ic | | | | 0.00 | 6.58 | | | | | 6.581 |
| 10.00 | 79,239 | 986.00 | 15.89 ic | | | | 0.00 | 15.89 | | | | | 15.89 |
| 12.00 | 99,415 | 988.00 | 76.11 ic | | | | 65.38 s | 10.72 s | | | | | 76.11 |

